Wastewater Basin Closure Plan

Intermountain Generating Facility

November 19, 2020

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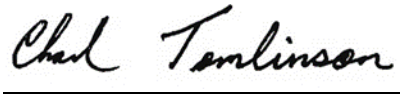
Approved by  _____
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Abbreviations

amsl	Above Mean Sea Level
ASTM	American Society for Testing and Materials
the Basin	Wastewater Basin
bgs	Below Ground Surface
CCR	Coal Combustion Residual
ft	Feet
HDPE	High-Density Polyethylene
IPA	Intermountain Power Agency
IPP	Intermountain Power Project
IPSC	Intermountain Power Services Corporation
L.L.	Liquid Limit
LLDPE	Linear Low-Density Polyethylene
N.P.	Non-Plastic
Plan	Closure Plan
P.I.	Plasticity Index
P.L.	Plastic Limit
TDS	Total Dissolved Solids
UAC	Utah Administrative Code Rule
UDEQ	Utah Department of Environmental Quality

1.0 INTRODUCTION

This Closure Plan (Plan) has been prepared to describe the activities which will be performed to obtain final closure of Intermountain Power Services Corporation's (IPSC's) Intermountain Power Project (IPP) Wastewater Basin (the Basin). The site is located approximately ten miles north of Delta, Utah. The Basin has been used to store coal combustion residual (CCR) material, predominantly thickener overflow, which is a slurry comprised of fine particles and water. This material is hydraulically conveyed to the Basin and stored in a saturated condition.

This Plan has been prepared for IPSC by Stantec for review and approval by the Utah State Department of Environmental Quality (UDEQ) Division of Waste Management and Radiation Control.

1.1 PURPOSE AND SCOPE

The CCR waste contained in the Basin could pose both a long-term source of fugitive dust emissions and a potential contamination source to groundwater. Therefore, the purpose of this document is to present the closure plan to eliminate fugitive dust emissions and potential groundwater impacts from the Basin in compliance with applicable regulatory requirements.

This document provides a detailed description of the activities to be performed as part of the proposed Plan – to close and cover the Basin with the CCR waste in place. These activities include:

- Removal of standing water.
- Construction of a divider berm to facilitate staged construction of the cover system over the CCR.
- Placement of pre-loading material (general fill) to promote consolidation and dewatering of the CCR.
- Dewatering of WW Basin solids.
- Construction of a sloping crown for the final cover system.
- Placement of a linear low-density polyethylene (LLDPE) liner to prevent surface water infiltration.
- Capping with 18-inches of general fill to protect LLDPE liner.
- Construction of a 6-inch layer of topsoil and seeding to promote vegetation growth.

The final cover system presented in this Plan will utilize LLDPE liner, overlain by an 18-inch liner protection layer and a 6-inch topsoil layer. The final cover system is designed to achieve the following:

- Provides a cover system that has a permeability less than or equal permeability rate of the bottom liner system of the CCR unit.
- Minimizes infiltration into the underlying CCR waste material.
- Provides an erosion protection layer consisting of a topsoil and vegetation.

This cover system has been designed to meet the Utah Administrative Code Rule (UAC) R315-319-102(d)D3) regulations for Closure and Post-Closure Care – Criteria for Conducting the Closure or Retrofit of CCR Units as discussed in Section 2.1.2.

In addition, a post closure monitoring plan has been designed to monitor the performance of the proposed Plan.

2.0 PROJECT BACKGROUND

The IPP is a 1,900-megawatt coal-fired, steam electric generation station located on an approximately 4,600-acre site in the Sevier Desert approximately 10-miles North of Delta, Utah. The IPP is owned by the Intermountain Power Agency (IPA) and operated by IPSC. The IPP began generating power in 1986 and has operated continuously since that time. The IPP delivers power to users located in Utah and Southern California. In May 2017, IPSC announced plans to cease power generation using coal and to develop new, natural-gas fueled generation at the project site by 2025. As a result of this transition, there are several CCR units at the plant that must be closed.

An Initial written closure plan (Stantec, 2016) was developed in 2016 to comply with UAC R315-319-102(b) that requires IPSC to submit a written closure plan to the Division of Waste Management and Radiation Control. The basis of the 2016 closure plan was that the existing CCR material would be left in place.

2.1 APPLICABLE REGULATORY REQUIREMENTS

2.1.1 UDEQ Requirements

A review of current UDEQ regulations/guidelines was conducted by Stantec to determine if there is a presumptive requirement for closure of the Basin following cessation of its operation. The review identified the UAC R315-319-102 titled “Closure and Post-Closure Care – Criteria for Conducting the Closure or Retrofit of CCR Units”. These criteria have been in effect since September 1, 2016 (UDEQ, 2016). UAC R315-319-102 outlines the closure and post-closure process, minimum reporting, and performance criteria required for CCR impoundments. UAC R315-319-102 was used as a reference guideline during the development of the proposed Plan. Specifically, the UDEQ rule includes the following requirements, summarized in **Table 2.1**, for the closure of an inactive CCR surface impoundments:

Table 2.1 Closure Performance Standards When Leaving CCR In Place (R315-319.10(d))

Section R315-319.102(d)	Description of Requirement	Wastewater Basin Closure Design
(1)(i)	Control, minimize, or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere	The LLDPE liner will act to prevent infiltration of liquids into the Basin and prevent surface water from contacting the CCR.
(1)(ii)	Preclude the probability of future impoundment of water, sediment, or slurry	The cover and surrounding area will be graded to shed stormwater away from the cover. Diversion channels will be maintained

Section R315-319.102(d)	Description of Requirement	Wastewater Basin Closure Design
		upstream of the Basin to prevent run-on from precipitation.
(1)(iii)	Include measures to provide for major slope stability to prevent the sloughing or movement of the final cover system during the closure and post-closure period	Dewatering the Basin and construction of a 3% graded crown to maintain positive drainage even after nominal amounts of settlement and/or subsidence has occurred.
(1)(iv)	Minimize the need for further maintenance of the CCR unit	The cover will be vegetated with a native seed mix. Once established, the cover will require little or no long-term maintenance.
(1)(v)	Be completed in the shortest amount of time consistent with recognized and generally accepted good engineering practices.	The construction of a divider berm will allow construction to occur on part of the Basin while deposition of CCR slurry can occur in other parts of the Basin.
(2)(i)	Eliminate free liquids by removal or solidifying remaining wastes and waste residues	Dewatering sumps will be excavated to remove water from the CCR deposit. General fill with a 3% gradient will be placed to partially stabilize and account for any nominal amount of settlement. The use of LLDPE liner accommodates larger differential settlement then high density polyethylene (HDPE) liners; therefore, it is preferred to HDPE. .
(2)(ii)	Stabilize remaining wastes to sufficiently support final cover system	
(3)(i)(A)	The permeability of the final cover system shall be less than or equal to the permeability of any bottom liner system or natural subsoils present, or a permeability no greater than $1x10^{-5}$ cm/sec, whichever is less.	The Basin cover system incorporating a LLDPE geomembrane is equivalent to the basin bottom HDPE liner.
(3)(i)(B)	The infiltration of liquids through the closed CCR unit shall be minimized by the use of an infiltration layer that contains a minimum of 18 inches of earthen material.	The low permeability layer consists of a LLDPE liner overlain by an 18-inches thick layer of general fill to provide liner protection and to minimize the infiltration of surface water.
(3)(i)(C)	The erosion of the final cover system shall be minimized by the use of an erosion layer that contains a minimum of six inches of earthen material that is capable of sustaining native plant growth.	The design of the soil cover includes a 6-inch thick erosion protection layer. The topsoil layer will be fertilized and seeded with a native seed mix to establish vegetation and protect against erosion.
(3)(i)(D)	The disruption of the integrity of the final cover system shall be minimized through a design that accommodates settling and subsidence	The Basin includes dewatering, preloading general fill and 3% gradient crown to accommodate nominal amounts of settlement. In addition, the inactive portion of the Basin will be surcharged early in the closure process to facilitate consolidation and dewatering of the CCR material.

Source: Utah Administrative Code Rule R315-319 (UDEQ, 2020)

2.1.2 Performance Standards for Surface Impoundment Covers

The UDEQ final rule for disposal of coal combustion residuals (UDEQ, 2016) requires that the permeability of the cover surface for Basin be less than or equal to the permeability of the bottom liner, or 1 x 10^{-5} cm/sec, whichever is less. The existing liner system beneath Basin is an 80-mil thickness high density polyethylene (HDPE) geomembrane on top of a prepared subsoil bedding surface. The liner system was initially constructed in 1986.

As the existing liner system beneath the Basin is an high density polyethylene (HDPE) liner it is necessary the cover system incorporates a liner system of similar permeability. An LLDPE geomembrane is incorporated in the cover system due to its elongation properties, allowing for differential settlements to occur while being resistant to impact and puncture. The LLDPE liner meets the UDEQ requirement of equal permeability of the bottom liner and is appropriate for use in the cover system.

3.0 SITE CONDITIONS

This section presents a summary of the Basin's characteristics as well as a description of the geological and hydrogeological conditions at the site. The majority of this information has been obtained from the Coal Combustion Residual Units Initial Closure Plan (Stantec, 2016), Specific Site Assessment for Coal Combustion Waste Impoundments at Intermountain Generating Station (GEI, 2011), and IPP Coal Combustion Waste Ponds – Geotechnical Stability Analysis Report (Gerhart Cole, 2013).

3.1 WASTEWATER BASIN DESCRIPTION

The Basin was commissioned in 1986. The major waste sources to the Basin is flue gas emissions control residuals. The Basin was designed and constructed with an 80 mil HDPE liner to minimize seepage from the basin.

Figure 3-1, the Basin current conditions. The Basin contains approximately 1,300,000 CY of waste, covering an area of approximately 51 acres at closure (Stantec, 2016). It is impounded by approximately 6,000 feet of perimeter berm, approximately 15 feet high, with a crest width of approximately 20 feet, and primarily constructed below native ground. The top of the berm is elevated slightly above the surrounding ground elevation to prevent surface water run-on into the Basin. The Basin bottom elevation is 4,630.0 feet (ft) above mean sea level (amsl) and the top of the basin berm is at elevation 4,650.0 ft amsl (20 ft depth). Refer to Figure 3-2 for a typical cross-section of the berm. The upstream and downstream berm side slopes are 3H:1V. The existing operating procedures requires that a minimum pond freeboard depth of three (3) ft be maintained to provide adequate storage for the 50-year, 24-hr storm event (Stantec, 2016).

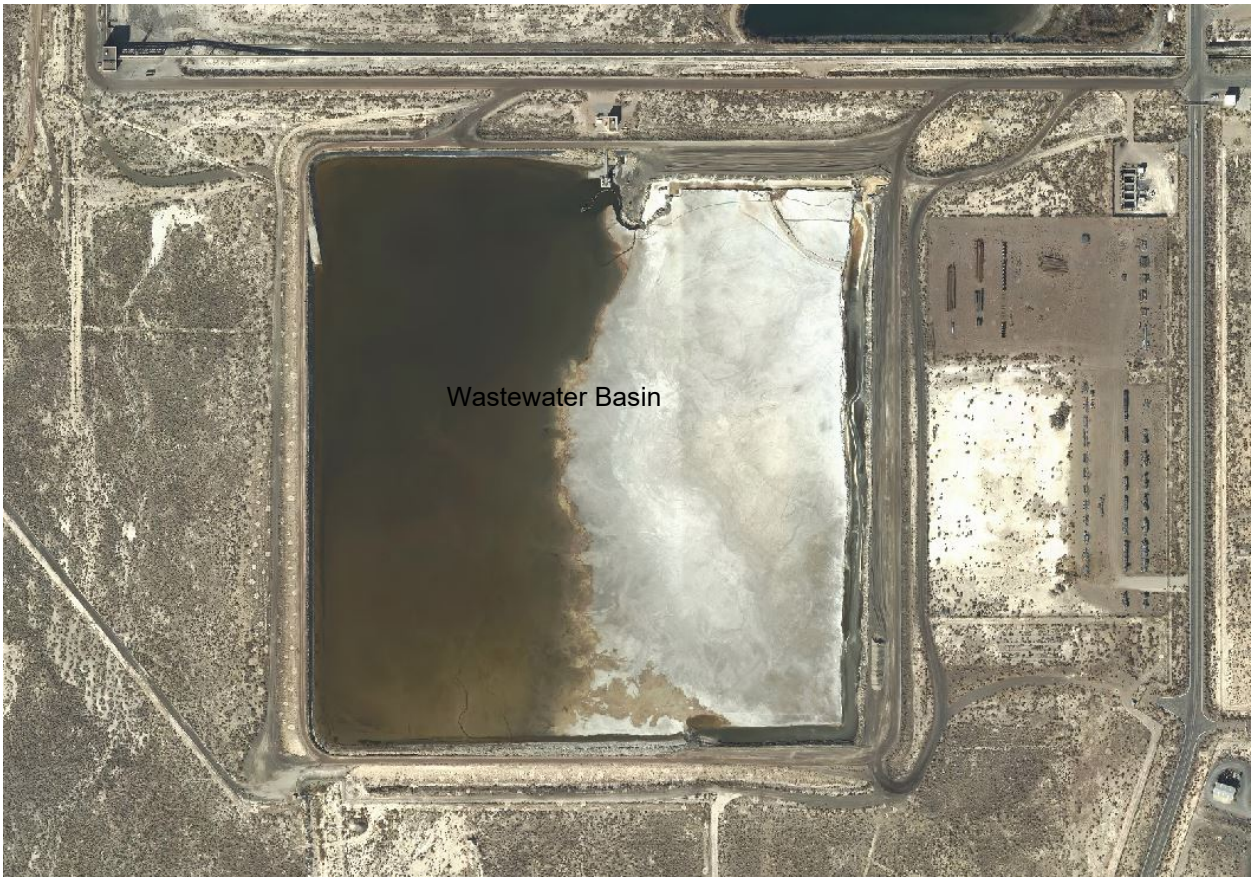

Figure 3-1 – Wastewater Basin Current Conditions

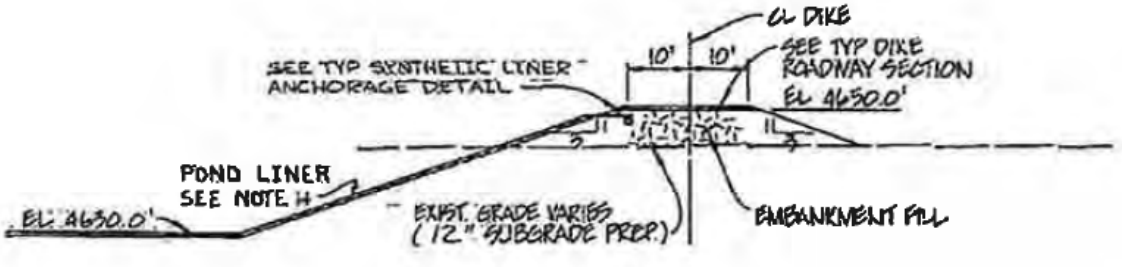

Figure 3-2 – Existing Typical Wastewater Basin Cross-Section

Based on correspondence with IPSC staff, only a portion of the Basin will need to remain active to meet the needs of the operational period of the Plant. The area to remain active is north of dividing berm on Sheet 410 of **Appendix A**. A large portion of the Basin, south of the dividing berm on Sheet 410 of **Appendix A**, is currently storing wastewater but could begin dewatering once the dividing berm has been constructed.

3.2 SITE GEOLOGY

The Wastewater Basin is near the center of the northern Sevier Desert in the Basin and Range Physiofgraphic Province (Figure 3-3). The area encompassing the Basin is located in the Sevier Lake drainage system. The ground surface within this area is relatively flat, sloping only slightly to the west. No major drainages cross the area.

The geology of the site is primarily comprised of interbedded lenses of sand and silty sand overlying lacustrine deposits. These surficial sediments are approximately 20 feet thick. The top few feet of the deposit are comprised of eolian sand, fluvial sand, and fine gravel. The underlying unit consists of fine-grained silts and clays of lacustrine origin. This unit is thickly bedded and extends to a depth of at least one hundred feet below ground surface (bgs). Both of the two major subsurface units dip slightly toward the west, paralleling the existing topographic slope.

Figure 3-3 – IPP Physiographical Location

3.3 GROUNDWATER

Groundwater levels underlying the Basin indicate a relatively flat groundwater surface roughly paralleling the ground surface. The average groundwater surface gradient is about 0.5 percent to

the west-southwest. The depths of the groundwater surface in the area range between 20 to 30 ft bgs.

Groundwater levels are measured and recorded semi-annually from 37 wells at the site as part of the Plant groundwater monitoring program (Stantec, 2020). The results of the groundwater monitoring program are documented in annual groundwater monitoring reports which are submitted to UDEQ. A brief description of groundwater conditions in the vicinity of the Basin are summarized here from the *June 2020 Semi-Annual Progress Report* (Stantec, 2020).

Based on measurements collected in March 2020, groundwater elevations in the vicinity of the Basin range between 4634.61 ft amsl (up-gradient of the Basin) and 4620.88 feet amsl (down-gradient of the Basin). The groundwater flow direction at the site is predominantly from northeast to southwest (Stantec, 2020).

The monitoring wells and associated groundwater elevations in the vicinity of the Basin that were sampled as part of the June 2020 Semi-Annual Progress Report (Stantec, 2020) are presented in **Table 3.1**.

Table 3.1 Representative Wells for Wastewater Basin

Well I.D.	Location	Groundwater Elevation (ft amsl) March 2020	Depth to Groundwater (ft bgs) March 2020
WW-U-1	Northeast of Wastewater Basin	4634.61	30.42
RW-4	Northwest of Wastewater Basin	4621.28	19.80
WWC-5	Northwest of Wastewater Basin	4621.36	20.39
WWC-4	West of Wastewater Basin	4621.16	19.42
WWC-3	Southwest of Wastewater Basin	4620.88	18.02
WWC-2	South of Wastewater Basin	4621.31	23.80
WWC-1	Southeast of Wastewater Basin	4621.87	22.85
WWC-8	Southeast of Wastewater Basin	4620.69	27.11
WWC-11	South of Wastewater Basin	4619.91	22.01

Groundwater data for the Basin indicates that most wells show little seasonal water level variation.

Water quality is monitored semi-annually at the Plant. During each sampling event, groundwater samples are collected from the representative wells listed in Table 3.1. All the groundwater samples are analyzed for water quality parameters.

As reported to the UDEQ in the past, and as is the current status based upon existing information, the plume of groundwater containing total dissolved solids (TDS) concentrations in excess of background concentrations is located within the uppermost aquifer beneath the IPSC-owned lands. The TDS plume is positioned well within the physical confines of IPSC-owned property and as such poses minimal risk to potential off-site receptors. The plume monitoring and corrective

actions being taken by IPSC are addressed in the Updated Corrective Action Plan (Stantec, 2016).

4.0 CLOSURE DESIGN

The following sections contain an overview of the anticipated closure activities for the Basin. Design drawings are presented in **Appendix A** and construction specification are provided in **Appendix B**. The regulations described in Section 2.1 were used as guidance for this closure design. The recommended closure has been chosen to achieve the following performance objectives:

- Provides a cover system that has a permeability less than or equal permeability rate of the bottom liner system of the CCR unit.
- Minimizes infiltration into the underlying CCR waste material.
- Provides an erosion protection layer consisting of a topsoil and vegetation.

4.1 CLOSURE STEPS

The closure of the Basin will be completed in steps as described in the following sections. The purpose of implementing the closure of the Basin in steps is to achieve the following:

- Bifurcate the Basin into operating and non-operating portions by construction of a divider berm to reduce the footprint of the Basin receiving CCR material.
- Allow for pumping of standing water on the southern (non-operating) portion of the Basin to begin to dry the CCR solids in this area.
- Allow for placement of fill on the southern (non-operating) portion of the Basin to allow for pre-consolidation of the CCR solids prior to installation of the final cover system to mitigate the potential for differential settlement of the final cover system.
- Allow for pumping of standing water on the Northern (operating) portion of the Basin to begin to dry the CCR solids in this area.
- Allow for placement of fill on the Northern (operating) portion of the Basin to allow for pre-consolidation of the CCR solids prior to installation of the final cover system to mitigate the potential for differential settlement of the final cover system.
- Install LLDPE liner cover over 3% sloped crown.
- Construction of an 18-inch soil protection layer.
- Construction of an 6-inch topsoil / vegetation layer.
- Monitor closure performance.

The closure design is presented in **Appendix A** and the major Plan activities are described in the following subsections and construction specifications are provided in **Appendix B**.

4.1.1 Step 1 Divider Berm Construction

The first step of the closure will involve constructing a divider berm to bifurcate the Basin into operating and non-operating cells. To facilitate the construction of the divider berm, the standing water, located predominantly in the western half of the Basin (refer to **Figure 3-1**) will be pumped to the existing evaporation ponds. The divider berm is shown on Sheet C-410 of **Appendix A.** The berm crest will be 100-feet wide and will extend at least 5-feet above the level of the existing Basin solids as show by Section A on Sheet C-411 of **Appendix A**.

The divider berm shall be constructed out of clay materials from Borrow Area 3 to limit migration of free water from the operating cell to the non-operating cell. The divider berm will likely be constructed starting at the east embankment, where the Basin solids are already dry at the surface and advance towards the west embankment. A 5-foot deep minimum cut-off trench will be constructed and backfilled with clay material from Borrow Area 3 (shown on Sheet C-410 & C-411 of **Appendix A**) in the western portion of the WW Basin to further preclude water from entering the non-operating cell. The floor of the clay trench shall be sloped towards the west side of the Basin to aid dewatering efforts and construction of the divider berm. Material cut for the clay trench shall be deposited in the inactive portion of the Basin. Placement specifications for the divider berm are presented in **Appendix B**.

4.1.2 Step 2 Initial Fill Placement and Dewatering of Southern Portion Basin Solids

Following pumping of the standing water and construction of the divider berm, fill placement on the southern (non-operating) portion of the Basin will begin. The initial fill placement design has been developed to facilitate achievement of the required slope and grade for placement of the LLDPE liner. Initial fill placement on the southern (non-operating portion) of the Basin well in advance of constructing the final cover system provides the following benefits:

- Allows for pre-loading and consolidation of the Basin solids to reduce the amount of settlement that is anticipated following installation of the final cover.
- Consolidation of the Basin solids will facilitate dewatering of the material.

The fill placement on the southern portion shall generally be advanced from the east side of the divider berm towards the dewatering sump / pumping area located in the southwest corner of the Basin. Advancement of fill in this manner will force the water in the Basin solids towards the dewatering sump. Water collected in the dewatering sump will be pumped, as needed, to the active (northern) portion of the Basin or to the existing evaporation ponds.

At the end of Step 2, the fill/subgrade will primarily match the required lines and grades required for installation of the final cover; however, the area encompassing the dewatering sump will remain open to accommodate continued dewatering.

As part of the initial fill placement, any Basin solids currently located outside the actual footprint of the Basin will be placed into the basin prior to that area receiving fill.

Soil from the onsite Borrow Area 3 will be used to construct the pre-load fill. Although constructed of the same material as the cover soil layer, the soil placed as part of the pre-load fill (general fill) will be compacted at a higher density to reduce the potential of settlement within the soil cover itself. The pre-load fill will be constructed to the design grades as shown in the design drawings presented in **Appendix A**. Placement specifications for the general fill are presented in **Appendix B**.

4.1.3 Step 3 Initial Fill Placement and Dewatering of Northern Portion of Basin

Step 3 will involve fill placement and dewatering of the northern portion of the Basin once discharges from the Plant to the Basin have ceased and the inlet has been capped to preclude any further discharge of CCR material. Prior to placement of fill, standing water in the northern portion of the Basin will be decanted and pumped to the existing evaporation ponds. Similar to Step 2 for the southern portion of the Basin, fill placement will commence to consolidate the solids in a manner that will promote water to be forced to the northwest corner of the Basin. The water will then be pumped from a dewatering sump to the evaporation ponds. The fill/cover subgrade placement will be constructed to the design grades as shown in the design drawings presented in **Appendix A**. Placement specifications for the general fill are presented in **Appendix B**.

4.1.4 Step 4 Final Subgrade Grading

The dewatering sumps shall remain open to support dewatering until instructed by the Engineer. Once the dewatering sumps are no longer required, these areas shall be backfilled with general fill to the designed grades shown in the design drawings presented in **Appendix A**.

The final fill/subgrade surface to receive the LLDPE will be smooth and free of all rocks, stones, sticks, roots, sharp objects, or debris of any kind. The surface will be a firm, unyielding foundation for the membrane with no sudden, sharp, or abrupt changes or break in grade. Placement specifications for the general fill and prepared liner subgrade are presented in **Appendix B**.

4.1.5 Step 5 Final Cover Construction

The cover system has been designed to minimize infiltration of precipitation control runoff, sustain native vegetation, and minimize erosion. The final cover system will require minimal maintenance. The cover system will consist of a LLDPE liner barrier overlain by a soil cover (general fill) and a vegetated erosion control layer (topsoil).

4.1.5.1 Linear Low-Density Polyethylene (LLDPE) Liner Installation

Due to the low-strength characteristics of the Basin solids, there is a potential for differential settlement following construction of the final cover. Placement of pre-load fill and dewatering of the CCR solids in advance of the cover placement will mitigate post-cover installation settlement. However, ongoing settlement of the CCR will be unpredictable and non-uniform so a LLDPE liner has been selected because it can accommodate large amounts of differential settlement without impacting performance. The LLDPE liner will be supplied by an approved manufacturer and installed in accordance with manufacturer's installation instructions and the project specifications which are provided in **Appendix B**.

4.1.5.2 Liner Protection Layer

An 18-inch thick liner protection layer will be installed over the liner. Soil used for this protective layer will be from the upper silty sand material within Borrow Area 3. Design drawings for the cover system are provided in **Appendix A**.

4.1.5.3 Erosion Protection Layer

Section 3(i)c of the UDEQ CCR Regulations states that "erosion of the final cover system shall be minimized by the use of an erosion layer that contains a minimum of six inches of earthen material capable of sustaining native plant growth." To provide an earthen material that promotes soil moisture storage and reduce the potential for soil erosion of the cover, this material will be a blend of clay and silty sand material obtained from Borrow Area 3.

4.1.5.4 Seedbed Preparation

Seedbed preparation and seeding will take place in the fall or early spring after grading and topsoil placement is complete. Following final placement of the topsoil, it will be tilled to a depth of 6-inches by ripping, discing, or other approved method to loosen compacted soil and leave a roughened, friable surface. Slopes shall be tilled on the contour leaving furrows perpendicular to the slope where practicable to reduce erosion and improve water capture and retention within the topsoil. Soil furrows and roughness are planned to shelter the seeds from wind and reduce development of erosion features, as well as collect water needed for the seeds to germinate.

4.1.5.5 Seeding

Following tilling the seed mix will be seeded over the entire area. Seeding will occur in late Fall (mid-October or later) or in early Spring (before the first of May). Reclamation seed mixtures shall be similar to the native plant species found at the site. Seed mixture should provide forage and cover species, which are similar to pre-disturbance conditions.

Immediately following seeding, the site will be mulched with weed-free straw or hay at a rate of 2 tons/acre. The straw or hay will be crimped into the soil to secure the mulch and to reduce

movement by wind. Hydromulching with a wood fiber mulch may be used as an alternative to straw or hay and applied at a rate of 1.5 tons/acre along with a tackifier to bind the mulch to the soil.

Specifications for seeding and mulching of the soil cover are presented in **Appendix B**. If an alternative seeding method is utilized, IPSC will notify UDEQ and provide a modified seeding plan for the alternative method prior to commencing seeding operations. Reclaimed borrow areas will also be re-vegetated to control runoff, reduce erosion, and blend into the surrounding topography.

4.1.5.6 Seed Mix Design

Seed mix selection will be based on a combination of plant species, characteristics, and conditions at the site. Seed species selection criteria will be based on soil texture and chemistry, precipitation, temperature and growing season, seed availability, and ease of species establishment. The following recommendations should be used in determining the proposed seed mixture:

- Native Plants are better adapted to the harsh desert climate of central Utah.
- Seed mixture should reflect the type of plants that grew prior to disturbance.
- Seed should come from a similar elevation and latitude to the site.
- Seed should be applied at a seeding rate between 14 to 28 pure live seed (PLS) pounds per acre for drill seeding (rates may be higher for broadcast seeding).

The seed mix should be comprised of a variety of native shrubs, grasses and forbs to provide habitat diversity and maximize evapotranspiration at the site.

4.1.6 Stormwater Controls

There is currently no inflow into the Basin from any upstream catchments. Stormwater controls, to prevent surface water from entering the Basin, will not be required.

4.2 BORROW SOURCE INVESTIGATOIN

The borrow source planned for the divider berm, general fill and soil cover will be obtained from an approximate 59-acre area west of the Evaporation Ponds and labeled as Borrow 3 on Sheet G-003 of **Appendix A**. Borrow material characterization was completed by soil laboratory testing methods on samples collected from excavated test pits within the borrow area. Five-gallon bucket composite samples were collected for each material type encountered in each of the 3 test pits. The associated test pit logs and laboratory testing are provided in **Appendix C** and **Appendix D**.

4.2.1 Borrow Source Sampling

Following the collection of the composite samples from the test pits, the samples were sent to Intermountain GeoEnvironmental Services, Inc. (IGES) in Salt Lake City, Utah for geotechnical and hydrological testing. The testing program is summarized in **Table 4.1**.

Table 4.1 Geotechnical and Hydrological Testing

Test	ASTM Method	Number of Samples	Comments
Organic Content	D2974	1	1 per borrow source
Atterberg Limits	D4318 a	6	2 per test pit sample
USCS Classification	D2487	6	2 per test pit sample
Particle-Size Distribution	D6913	6	2 per test pit sample
Hydrometer Analysis	D7928	6	2 per test pit sample
Crumb Test	D6572	2	2 per borrow source
Standard Proctor	D698 b	1	1 per borrow source
Hydraulic Conductivity	D5084	1	1 per borrow source

Source: IGES Laboratory Testing Results (IGES, 2020)

The test results are summarized in Table **4.2**. Complete laboratory reports for the testing are presented in **Appendix D**.

Table 4.2 Geotechnical and Hydrological Testing on Composite Samples

Soil Test	B3TP1	B3TP1	B3TP2	B3TP2	B3TP3	B3TP3	Comb. B3 TP1-3
Composite Sample Depth	0-10’	10-20’	0-15’	15-25’	0-15’	15-30’	10-30’
USCS Classification	SC	CL	CL	CL	SM	CL	CL
Standard Proctor Compaction Test (MDD lbs/ft^3)	-	-	-	-	-	-	105.4
Optimum Moisture Content (OMC%)	-	-	-	-	-	-	20.8
Particle Size Distribution %Gravel %Sand %Fines	3.3 69.3 27.4	0.9 23.6 75.5	0.8 32.7 66.5	1.3 30.1 68.6	2.1 73.6 24.3	0.8 20.6 78.6	-
Atterberg Limits LL[a/] (%) PL[b/] (%) PI [c/] (%)	29 17 12	34 15 19	31 14 17	30 15 15	N.P.[d/]	29 15 14	
Organic Matter (%)		3.2					

Crumb Test[e]				Grade 1	Grade 1		
Average Hydraulic Conductivity K (cm/sec)							1.5E-05

Notes:

[a]/ LL: Liquid Limit

[b]/ PL Plastic Limit

[c]/ PI: Plasticity Index

[d]/ **N.P.: Non-Plastic**

[e]/ Crumb Test Results: Grade 1 – Nondispersive

5.0 POST CLOSURE OPERATION AND MAINTENANCE PLAN

This section describes activities that will be conducted as part of the post-closure process. Utah Administrative Code Rule R315-319-104 titled Closure and Post-Closure Care – Post-Closure Care Requirements (UDEQ, 2016) require that a post-closure plan be developed and implemented for a period of 30 years once closure and reclamation activities have been completed. Post-closure is the process which is used to allow a facility to stabilize to the point where it no longer presents a threat to human health or the environment. During this period, the facility will be routinely monitored to ensure that the integrity of the soil cover is not compromised by erosion and settlement and ensure that the soil cover's performance is acceptable. Therefore, this post-closure plan will provide the following:

- A plan for inspection and maintenance of the soil cover.
- A description of the proposed use of the property during the post-closure care period.

IPSC may petition for the UDEQ to terminate the post-closure period earlier if they can demonstrate that the soil cover has stabilized and is protective of groundwater.

5.1 COVER INTEGRITY MONITORING AND MAINTENANCE

Following construction of the soil cover, routine monitoring will be performed to identify the need for maintenance of the soil covers. The monitoring will include both visual inspection and surveying of the soil cover to ensure that their integrity is not being compromised. The monitoring plan, including the individual monitoring tasks, inspection locations, schedule, monitoring criteria, and possible maintenance is summarized in **Table 5.1**.

Table 5.1 Post-Closure Monitoring Summary

Monitoring Activity	Purpose	Monitoring Frequency	Monitoring Locations	Monitoring Method	Comments	Actions Items
Visual Cover Inspection	Visually inspect soil cover surface for ponding, sags, drainage interruptions, surface erosion, and vertical cracking.	Semi-Annually and Following major storm events of 1-inch or more of rainfall in 24-hrs.	Entire Basin area.	Visual	The locations of ponding, sags, drainage interruptions, surface erosion, and vertical cracking shall be noted on the inspection form.	Ponding, sags, and drainage interruptions will be repaired, regraded, and re-vegetated.
Vegetation Inspection	Inspect soil cover for vegetation establishment.	Semi-Annually	Entire basin area.	Visual	Any areas showing vegetation distress such as bare areas or significantly lower vegetative establishment compared to rest of the soil cover will be noted on the inspection form.	Bare areas will be repaired during the next seeding season.
Groundwater Monitoring	Detect potential migration of spent liquor from the Wastewater Basin.	Semi-Annually	In accordance with the approved groundwater monitoring well list for the Plant.	In accordance with the approved groundwater monitoring parameter list for the Plant	None	Record significant deviations in groundwater quality to UDEQ.

5.1.1 Visual Cover Inspection

Visual inspections of the soil cover will be performed to identify damage to or degradation of the soil cover including; the formation of rills, loss of vegetation over significant portions of the soil cover, and formation of visible animal burrows or trails over the soil cover. The visual inspections will be performed across the entire soil cover. Visual inspections of the soil cover will be performed semi-annually and following major storm events. The results of the visual inspections will be documented in site inspection reports and retained on-site for UDEQ review upon request.

5.1.2 Vegetation Monitoring

During the semi-annual soil cover monitoring, the cover vegetation will be inspected for burned areas, overall establishment, disease or pests, and noxious weed infestations. The inspections

will be performed during the semi-annually visual inspection of the soil cover. Any areas showing vegetation distress such as bare areas or significantly lower vegetative establishment compared to rest of the soil cover will be clearly noted on the inspection form.

5.2 SOIL COVER MAINTENANCE

The purpose of the final cover maintenance procedures is to maintain the integrity of the soil cover over the long-term. Semi-annual visual inspections and settlement monitoring will provide identification of erosion and settlement. A site representative, designated by IPSC, will be responsible for documenting the location and extent of repairs.

All final cover repairs and/or reconstruction shall be conducted in a manner directed to maintain the integrity of the as-built final cover system. Repair of fill materials will be performed to maintain the 18-inch general fill and 6-inch erosion protection layers. The methods of repair will be performed for the following principal modes of final cover distress:

- Settlement related sags and drainage interruptions, which interfere with controlled flow and discharge of surface waters from the soil cover surface
- Surface erosion because of overflow associated with intense rains
- Local surficial slumping on slopes resulting from intense rains
- Vertical or near vertical cracking of cover soils as a result of settlement.

5.2.1 Depressions, Ponding, Drainage Interruptions and Surface Erosion

Any repairs of depressions in the final soil cover will be completed on an annual basis. If significant sags or ponding is identified during other times of the year, the IPSC representative will accurately locate the limits of the depressions. The IPSC representative will be responsible for directing fill placement in the sag area to reestablish the intended grade of the cover. The permanent repair of sags and ponding, when necessary, will be performed by adding sufficient cover soil material necessary to maintain the design slope. Cover soil will be placed in accordance with the design specifications. An IPSC representative shall inspect and certify any fill placed in the final cover layers. Repaired areas shall also be re-seeded in accordance with the design specifications.

5.3 POST-CLOSURE INSPECTION AND MAINTENANCE REPORTING

All copies of the operator's inspection and maintenance reports will be retained on-site for UDEQ review upon request to demonstrate that the site has been inspected on a routine basis to evaluate the integrity and stability of the soil cover and stormwater diversion systems. Any repairs or maintenance performed will be discussed in detail in maintenance reports.

5.4 GROUNDWATER MONITORING

The current groundwater monitoring and corrective actions being taken by IPSC are addressed in the Updated Corrective Action Plan (Stantec, 2016) and will be continued to be followed during the post-closure phase until conditions warrant revisions to the groundwater monitoring plan.

6.0 CLOSURE SCHEDULE

Per the requirements of UAC R315-319-102(b)(1)(vi), a preliminary closure schedule has been developed for the Basin. The schedule showing key construction milestones is presented in **Appendix E**. The schedule was developed based on the stepped closure approach discussed in Section 3 and was based on the following assumptions:

- Bifurcation for the Basin and fill placement and dewatering of the southern portion of the Basin will be conduct prior to the cessation of CCR flows to the northern portion of the Basin.
- Fill placement and dewatering of the northern portion of the Basin would commence following conversion to gas and cessation of flows, which is anticipated to be July 1, 2025.
- Prior to construction of the final cover system, the schedule included 100 days to complete dewatering of the Basin.
- Construction of the final cover system will on the northern and southern portions of the Basin will occur concurrently following fill placement and dewatering the northern portion of the Basin.
- Seeding of the final cover system was fixed to only occur in the late fall to improve vegetation establishment.

Based on the schedule developed, closure activities for the Basin are anticipated to be completed by November 5, 2026.

7.0 REFERENCES

GEI, 2011. Specific Site Assessment for Coal Combustion Waste Impoundments at Intermountain Generating Station. Delta, Utah. April 2011.

Gerhart Cole, 2013. IPP Coal Combustion Waste Ponds. Geotechnical Stability Analysis Report. April 2013.

IGES, 2014. Geotechnical Laboratory Testing Results – IPSC CCR Unit Closures, Delta, UT.

Stantec, 2016. Coal Combustion Residual (CCR) Units Initial Closure Plan. Intermountain Generating Facility. Delta, Utah. October 13, 2016.

Stantec, 2020. June 2020 Semi-Annual Progress Report. Intermountain Generating Facility. Delta, Utah. June 25, 2020.

UDEQ, 2016. R315. Environmental Quality, Waste Management and Radiation Control, Waste Management. R315-319. Coal Combustion Residual Requirements., Issued September 2016.

Appendix A

IPSC CCR Wastewater Basin Closure Design

PRELIMINARY DESIGN PHASE - 10/02/2020

NOT FOR CONSTRUCTION

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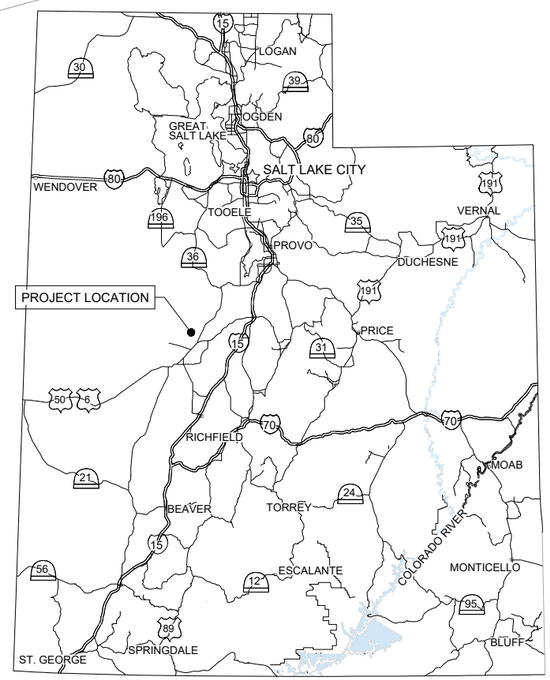
INTERMOUNTAIN POWER SERVICE CORP

Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN

CLOSURE DESIGN SUBMITTAL - OCTOBER 2020



AREA MAP
NTS

INDEX OF DRAWINGS	
DRAWING NO	DRAWING NAME
G-001	COVER SHEET AND DRAWING INDEX
G-002	GENERAL NOTES AND SYMBOLS
G-003	EXISTING SITE LAYOUT
C-400	WASTEWATER BASIN CLOSURE - EXISTING CONDITIONS
C-410	WASTEWATER BASIN CLOSURE - STEP 1 DIVIDER BERM LAYOUT
C-411	WASTEWATER BASIN CLOSURE - STEP 1 DIVIDER BERM SECTIONS
C-420	WASTEWATER BASIN CLOSURE - STEP 2 INITIAL FILL PLACEMENT AND DEWATERING OF SOUTH PORTION OF BASIN
C-421	WASTEWATER BASIN CLOSURE - STEP 2 INITIAL FILL PLACEMENT SECTIONS
C-430	WASTEWATER BASIN CLOSURE - STEP 3 INITIAL FILL PLACEMENT AND DEWATERING OF NORTH PORTION OF BASIN
C-431	WASTEWATER BASIN CLOSURE - STEP 3 INITIAL FILL PLACEMENT SECTIONS
C-440	WASTEWATER BASIN CLOSURE - STEP 4 SUBGRADE PLACEMENT
C-450	WASTEWATER BASIN CLOSURE - STEP 5 FINAL COVER DESIGN
C-451	WASTEWATER BASIN CLOSURE - STEP 5 FINAL COVER SECTIONS
C-460	WASTEWATER BASIN CLOSURE - STEP 5 FINAL COVER CONTROL POINTS
C-461	WASTEWATER BASIN CLOSURE - STEP 5 FINAL COVER CONTROL POINTS TABLE
C-470	WASTEWATER BASIN CLOSURE - BORROW SOURCE 3 - PHASE 1 EXCAVATION PLAN
C-471	WASTEWATER BASIN CLOSURE - BORROW SOURCE 3 - PHASE 1 EXCAVATION SECTIONS
C-472	WASTEWATER BASIN CLOSURE - BORROW SOURCE 3 - PHASE 1 STAGE STORAGE CURVE
C-480	WASTEWATER BASIN CLOSURE - DETAILS

DWG FILE: C:\pwwrkdir\dms40954\G-001.dwg
PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER

CIVIL GENERAL NOTES

GENERAL

1. THE CONTRACTOR SHALL TAKE ALL PRECAUTIONARY MEASURES NECESSARY TO PROTECT EXISTING IMPROVEMENTS WHICH ARE TO REMAIN IN PLACE FROM DAMAGE. ALL IMPROVEMENTS DAMAGED BY THE CONTRACTOR'S OPERATIONS SHALL BE EXPEDITIOUSLY REPAIRED OR RECONSTRUCTED AT THE CONTRACTOR'S EXPENSE WITHOUT ADDITIONAL COMPENSATION.
2. THE CONTRACTOR SHALL PROPERLY DISPOSE OF ALL DEBRIS FROM DEMOLITION AT CONTRACTORS EXPENSE.
3. CONTRACTOR SHALL RESTORE ALL SURVEY MONUMENTS THAT ARE DAMAGED OR DESTROYED DURING CONSTRUCTION.

UTILITIES

1. PRIOR TO THE START OF CONSTRUCTION, THE CONTRACTOR SHALL LOCATE ALL EXISTING UTILITIES IN AND AROUND THE AREAS OF NEW CONSTRUCTION.
2. THE CONTRACTOR SHALL PROTECT ALL REMAINING EXISTING UTILITIES.
3. LOCATIONS OF UNDERGROUND AND ABOVE GROUND UTILITIES SHOWN ON THE DRAWINGS WERE OBTAINED FROM AVAILABLE RECORDS. THE CONTRACTOR SHALL VERIFY ALL LOCATIONS AND ELEVATIONS AND SHALL TAKE ALL PRECAUTIONARY MEASURES NECESSARY TO PROTECT UTILITY LINES WHETHER SHOWN OR NOT SHOWN.
4. PRIOR TO ANY EXCAVATION IN THE VICINITY OF ANY EXISTING UNDERGROUND FACILITIES, INCLUDING ALL WATER, SEWER, STORM DRAIN, GAS, PETROLIUM PRODUCTS, OR OTHER PIPELINES; ALL BURIED ELECTRIC POWER, COMMUNICATIONS, OR TELEVISION CABLES; ALL TRAFFIC SIGNAL AND STREET LIGHTING FACILITIES; AND ALL ROADWAY, STATE HIGHWAY, AND RAILROAD RIGHTS-OF-WAY, THE CONTRACTOR SHALL NOTIFY THE RESPECTIVE AUTHORITIES REPRESENTING THE OWNERS OR AGENCIES RESPONSIBLE FOR SUCH FACILITIES NOT LESS THAN 3 DAYS NOR MORE THAN 7 DAYS PRIOR TO EXCAVATION SO THAT A REPRESENTATIVE OF SAID OWNERS OR AGENCIES CAN BE PRESENT DURING SUCH WORK IF THEY SO DESIRE. IN THE CASE OF THE UNDERGROUND UTILITY SERVICE ALERT CENTER, THIS NOTICE WILL GIVE THEM TIME TO MARK THE LOCATION OF THE UTILITIES. THE CONTRACTOR SHALL ALSO NOTIFY THE REGIONAL OR LOCAL UNDERGROUND SERVICE ALERT COMPANY AT LEAST 3 DAYS, BUT NO MORE THAN 7 DAYS, PRIOR TO SUCH EXCAVATION.

EROSION CONTROL

1. THE CONTRACTOR SHALL SUBMIT AN EROSION CONTROL PLAN FOR WORK DURING THE CONSTRUCTION, SIGNED AND STAMPED BY A REGISTERED CIVIL ENGINEER PRIOR TO THE START OF CONSTRUCTION.
   a. ALL SLOPES SHALL BE PROTECTED FROM EROSION DURING ROUGH GRADING OPERATIONS AND THEREAFTER, UNTIL INSTALLATION OF FINAL GROUNDCOVER.
   b. ALL SLOPE PROTECTION SWALES SHALL BE CONSTRUCTED AT THE SAME TIME AS BANKS ARE GRADED.
   c. THE CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTATION AND MAINTENANCE OF EROSION CONTROL MEASURES CONTAINED WITHIN THE CONTRACT SPECIFICATIONS. THE CONTRACTOR SHALL ALSO PROVIDE ANY ADDITIONAL EROSION CONTROL MEASURES (E.G. HYDROSEEDING, MULCHING OF STRAW, SAND BAGGING, DIVERSION DITCHES, ETC.) DICTATED BY FIELD CONDITIONS TO PREVENT EROSION OR THE INTRODUCTION OF DIRT, MUD, OR DEBRIS INTO EXISTING WATERWAYS, OR ONTO ADJACENT PROPERTIES DURING ANY PHASE OF CONSTRUCTION OPERATIONS.

CIVIL GENERAL NOTES - CONTINUED

SURVEY AND CONTROL

1. TOPOGRAPHY AND AERIAL IMAGERY BASED ON A NOVEMBER 2019 OLYMPUS AERIAL SURVEYS INC. SURVEY.
2. SURVEY IN LOCAL PLANT COORDINATE SYSTEM AND LOCAL DATUM IN INTERNATIONAL FEET.

PERMITTING

OWNER WILL BE RESPONSIBLE FOR OBTAINING PERMITS FROM THE UTAH DEPARTMENT OF ENVIRONMENTAL QUALITY.

GENERAL CIVIL SYMBOLS

- 4600 — MAJOR CONTOURS
- MINOR CONTOURS
- 4600 — EXISTING MAJOR CONTOURS
- EXISTING MINOR CONTOURS
- P/L — PROPERTY LINE
- R/W — RIGHT-OF-WAY LINE
- EASEMENT LINE
- TEMP ESMT — TEMPORARY EASEMENT LINE
- TRAIL OR DIRT ROAD
- FLOW LINE
- RAILROAD
- GUARDRAIL (PERMANENT)
- GUARDRAIL (REMOVABLE)
- VEGETATION
- WELL
- NEW
- EXISTING
- CENTERLINE
- 3:1 — BERM SLOPE (HORZ TO VERT)

GEOTECHNICAL SYMBOLS

- B-X — SOIL BORING LOCATION
- TEST PIT LOCATION
- XX — OBSERVATION HOLE
- MW — MONITORING WELL

STRUCTURES

- SITE OR RETAINING WALL
- X — X — FENCE (CHAINLINK)
- FENCE (WOOD)
- STRUCTURE
- STRUCTURE (BELOW GRADE)
- CONVEYOR SYSTEM

CONTROL SYMBOLS

- BM-XX — BENCH MARK
- ### — SITE COORDINATE NUMBER
- N XXXXXXX / E XXXXXXX — SITE COORDINATES
- EL XXXX.XX — MONUMENT
- HORZ AND VERT CONTROL POINT
- XXX.XX — FINISHED ELEVATION
- (XXX.XX) — EXISTING ELEVATION

REV 050215

REV	DATE	BY	DESCRIPTION
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A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: NTS

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DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
GENERAL
WASTEWATER BASIN CLOSURE
GENERAL NOTES AND SYMBOLS

SHEET: G-002

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM BY: WOOLSEY, ROGER

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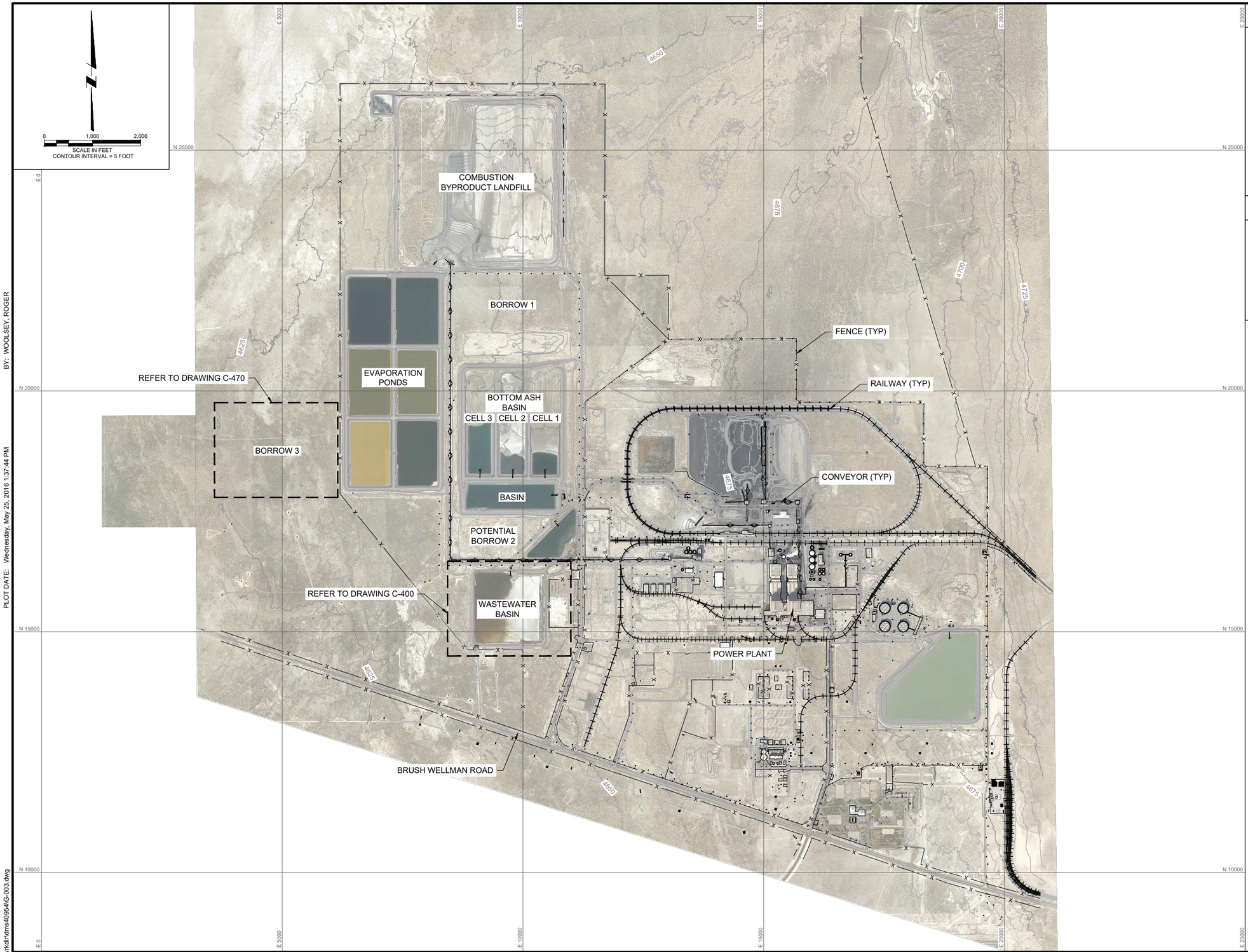

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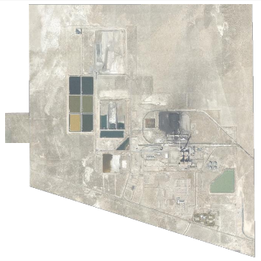

LEGEND

- EXISTING CONTOURS
- EXISTING RAILWAY
- CONVEYOR SYSTEM
- FENCE
- POWER POLE

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
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A	09/11/2020	CF	ISSUED FOR INTERNAL REVIEW

SCALE
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WARNING
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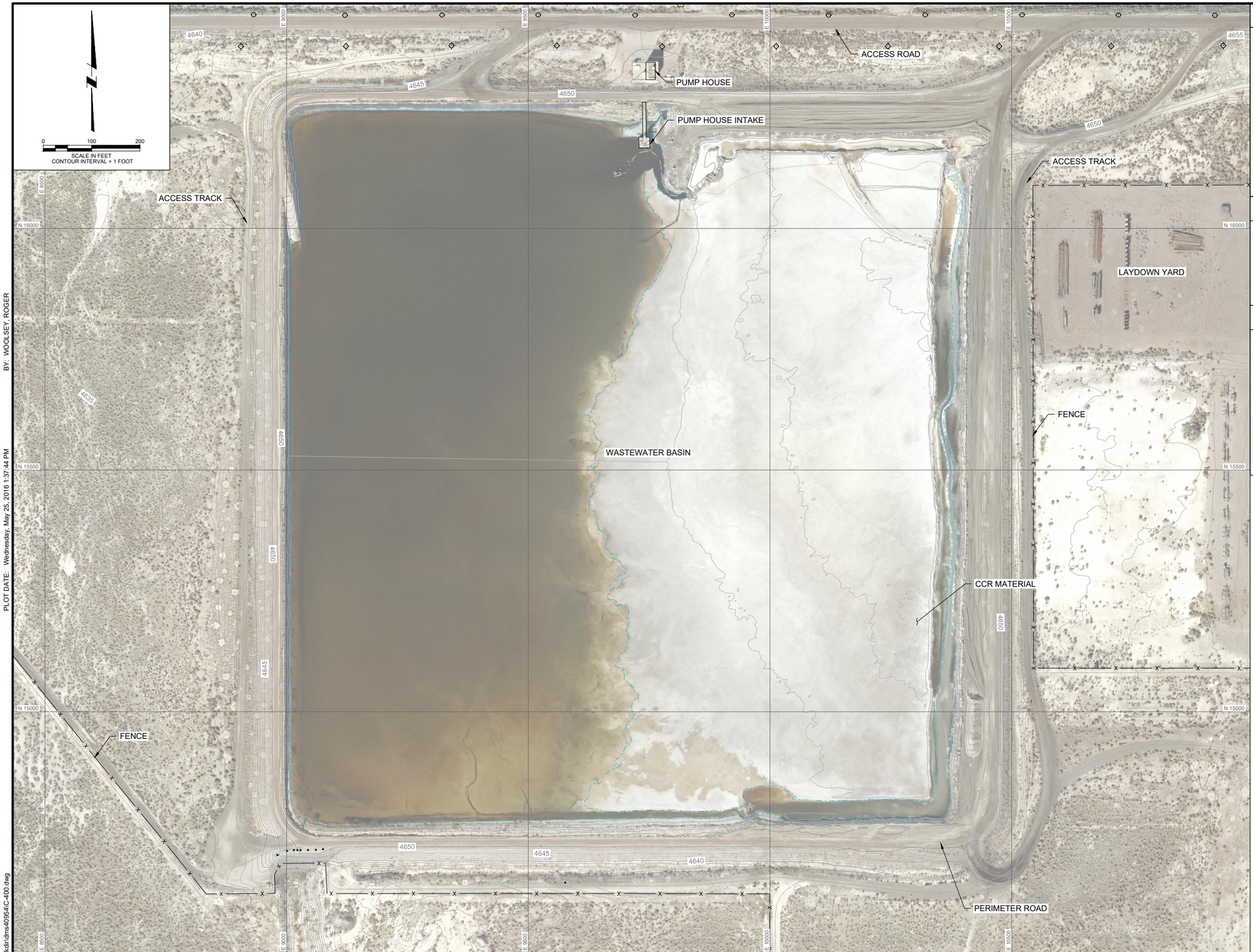
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
EXISTING SITE LAYOUT

SHEET
G-003

Job# 233001396



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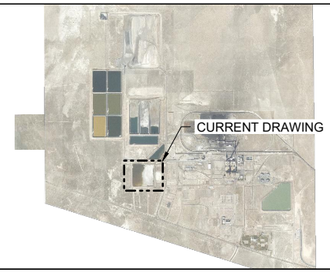

LEGEND

- 4600 — EXISTING CONTOURS
- x — FENCE
- WATER LEVEL
- POWER POLE

GENERAL SHEET NOTES

1. CONDITIONS OF WASTE WATER BASIN BASED ON SURVEY COMPLETED ON NOVEMBER 2019. ACTUAL EXTENT OF BASIN SOLIDS MAY VARY AT TIME OF CLOSURE.
2. ELEVATIONS OF SOLIDS WITHIN INUNDATED PORTION OF WASTE WATER BASIN ARE APPROXIMATE. CONTRACTOR SHALL VERIFY ELEVATION OF SOLIDS AND NOTIFY ANY DISCREPANCIES BETWEEN ACTUAL AND DESIGN ELEVATIONS PRIOR TO COMMENCING WORK.
3. PUMP HOUSE TO REMAIN OPERATIONAL AND CONTRACTOR SHALL PLAN THE SEQUENCING OF WORK WITH THE OWNER TO ALLOW FOR THIS OPERATION AND MAINTENANCE OF THE "ACTIVE" PORTION OF THE BASIN.

REV	DATE	BY	DESCRIPTION
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A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

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INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
EXISTING CONDITIONS

SHEET
C-400

Job# 233001396

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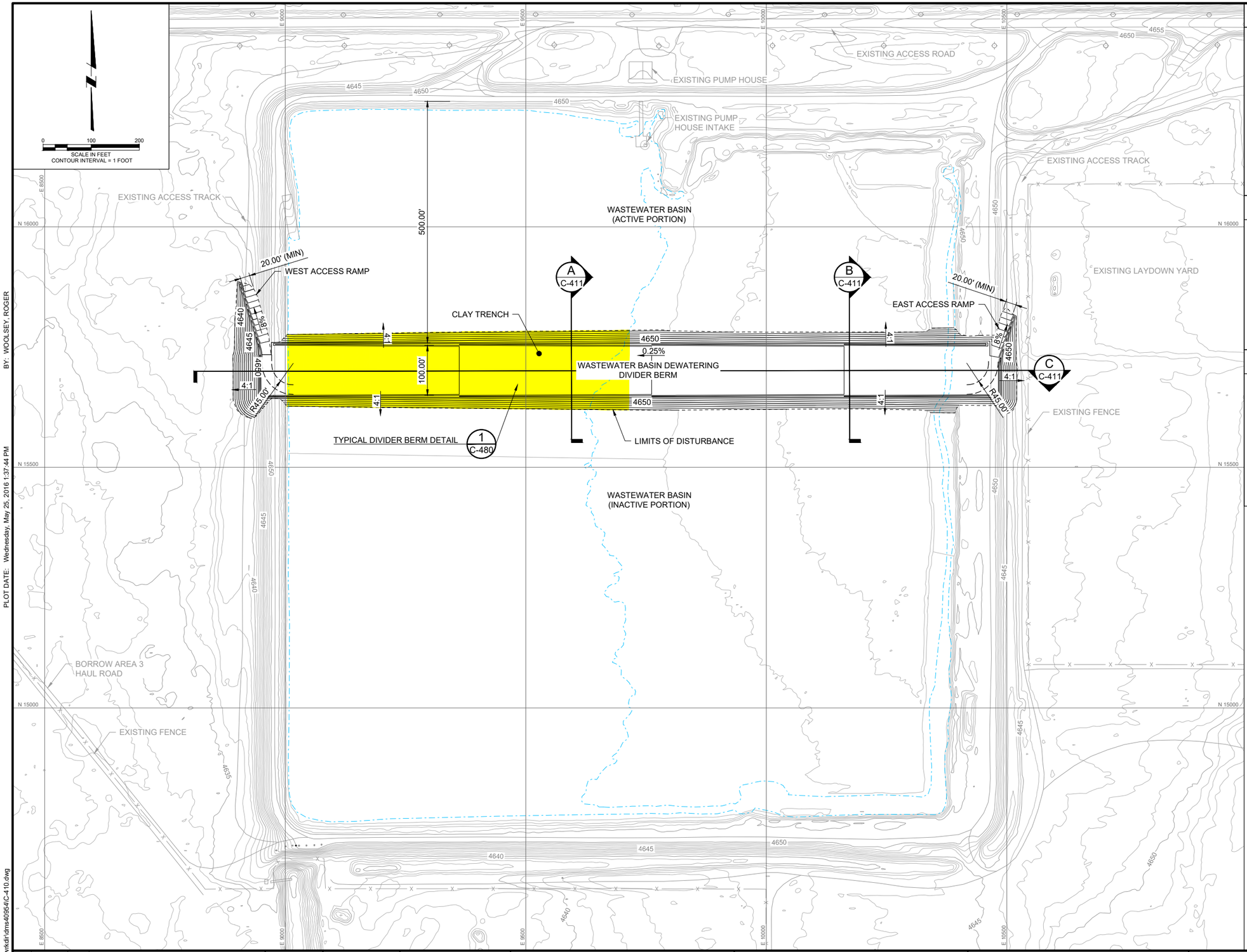

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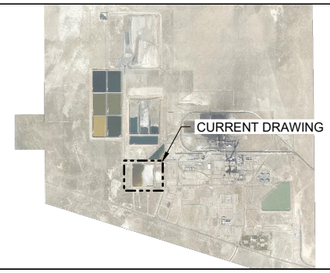

LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING FENCE
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- CLAY TRENCH

GENERAL SHEET NOTES

1. DIVIDER BERM CONSTRUCTED BY OTHERS.
2. SOUTHERN PORTION OF WASTEWATER BASIN WILL BE DECOMMISSIONED AND DEWATERED FIRST.
3. MATERIAL CUT FOR CLAY TRENCH SHALL BE DEPOSITED IN INACTIVE PORTION OF POND.
4. CLAY FOR DIVIDING BERM SHALL BE OBTAINED FROM BORROW SOURCE 3.
5. AREA ADJACENT TO DIVIDER BERM SHALL BE DEWATERED SUFFICIENTLY TO CONSTRUCT CLAY TRENCH WHERE NOTED.

QUANTITY TABLE

DESCRIPTION	CUT (CY)	FILL (CY)
CLAY TRENCH	27,410	-
CLAY DIVIDING BERM	-	73,660

REV	DATE	BY	DESCRIPTION
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A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

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DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

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INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 1 DIVIDER BERM LAYOUT

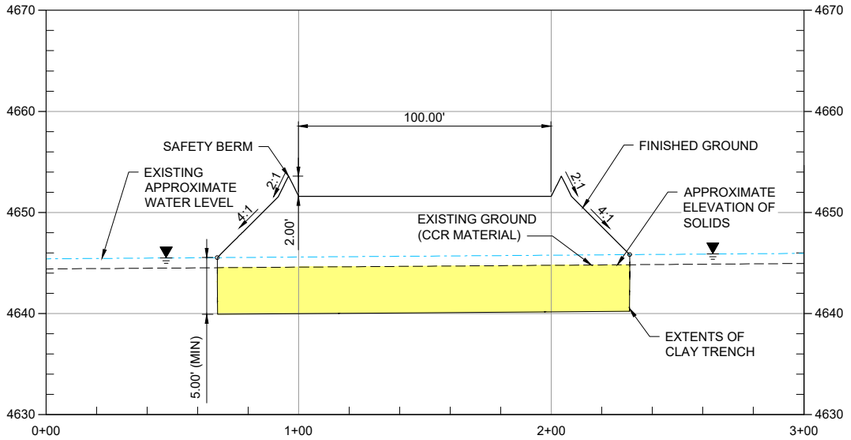
SHEET
C-410

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-410.dwg

GENERAL SHEET NOTES

1. FLOOR OF CLAY TRENCH TO BE SLOPED TOWARDS THE WEST TO FACILITATE CONSTRUCTION.
2. WATER PUMPED DURING DEWATERING SHALL BE PUMPED TO OPERATING PORTION OF WASTEWATER BASIN.
3. CONTRACTOR SHALL ENSURE PROTECTION OF EXISTING LINER WHEN EXCAVATING BASIN SOLIDS NEAR EMBANKMENT.



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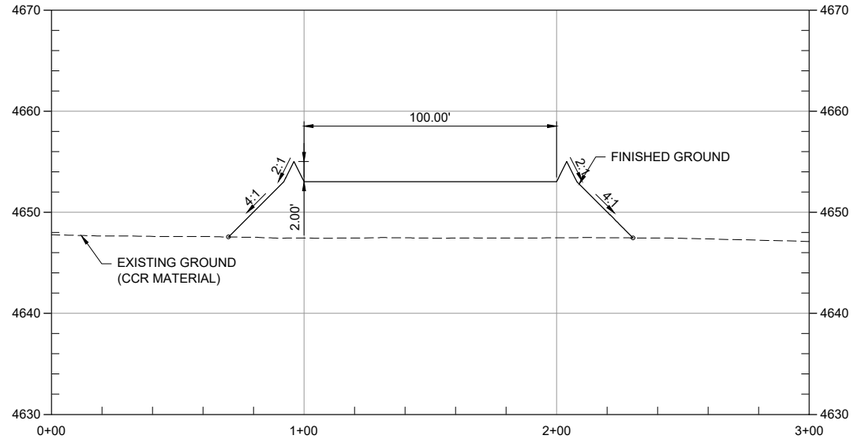
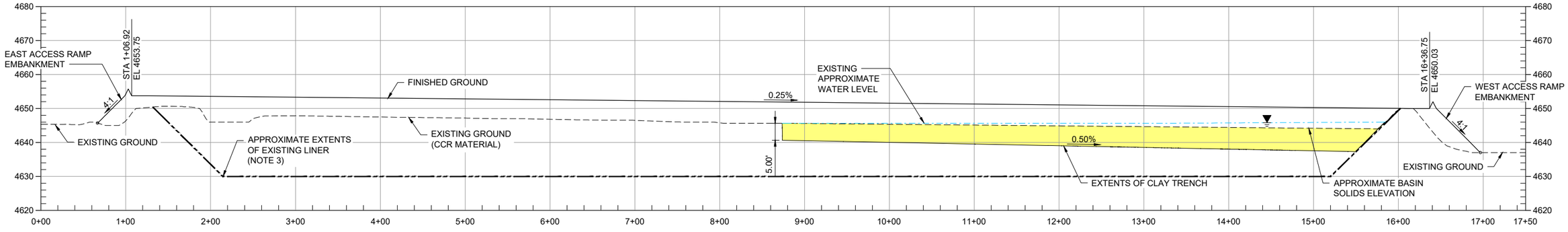


B C-410 SECTION B

HORZ 0 30 60
VERT 0 7.5 15
SCALE IN FEET



C C-410 SECTION C

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE 1"=100'

WARNING 0 ½ 1 IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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Stantec

INTERMOUNTAIN POWER SERVICE CORP

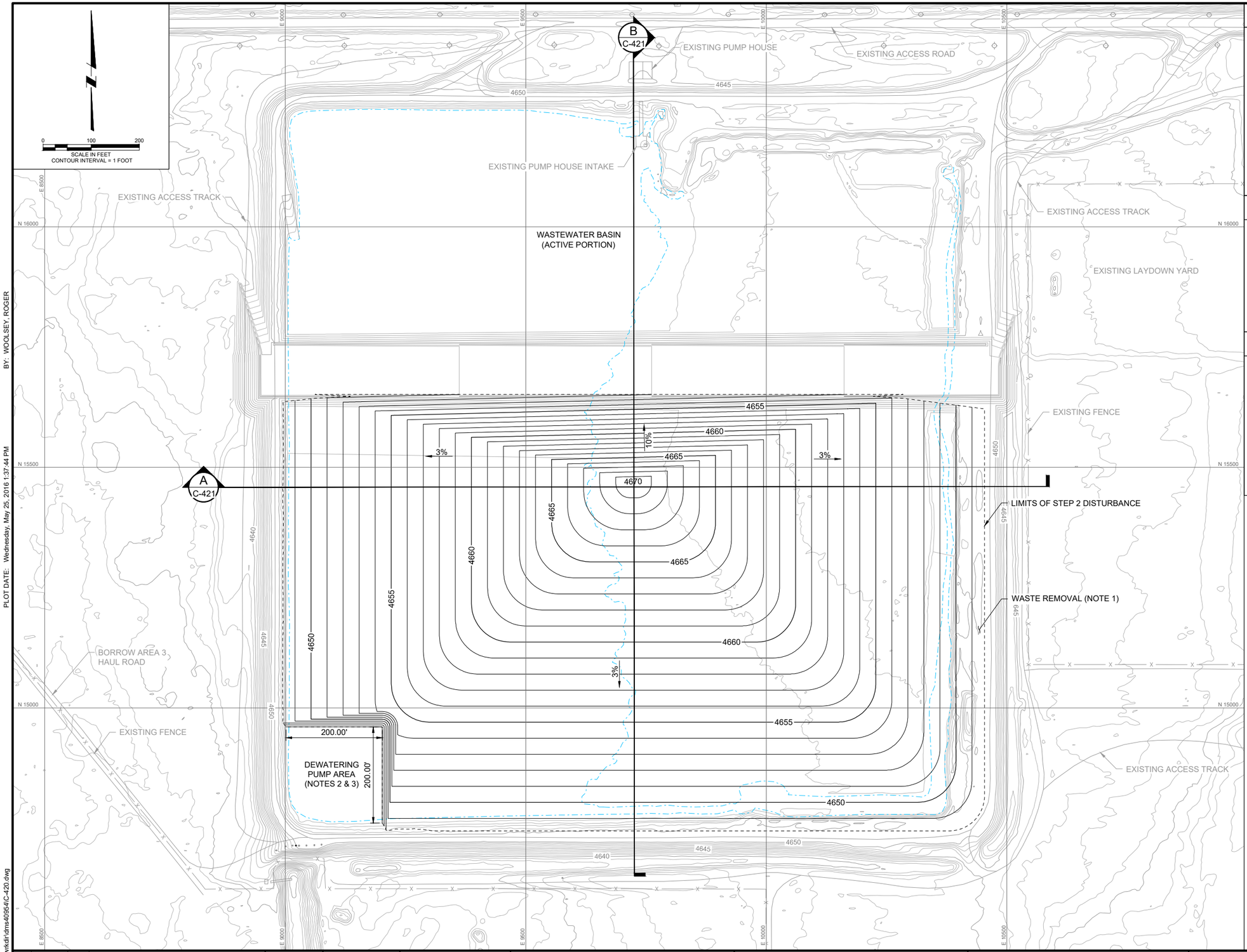
IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 1 DIVIDING BERM SECTIONS

SHEET C-411

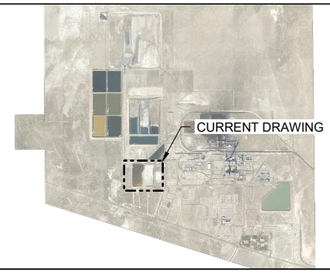
Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM BY: WOOLSEY, ROGER

DWG FILE: C:\pwwrkdir\dms40964\C-411.dwg



KEY PLAN



LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- EXISTING FENCE

GENERAL SHEET NOTES

1. WASTE SHALL BE REMOVED AROUND EXISTING EMBANKMENT (BY OTHERS) TO FACILITATE COVER SYSTEM CONSTRUCTION.
2. TEMPORARY SUCTION PUMP AND DISCHARGE LINES SHALL BE PROVIDED BY CONTRACTOR.
3. PUMPED WATER SHALL BE ROUTED TO THE EVAPORATION POND.
4. FILL QUANTITIES ARE BASED ON THE APPROXIMATE TOP OF WASTEWATER BASIN SOLIDS OF 4645 FT AMSL AND REPRESENT IN PLACE QUANTITIES.
5. DEWATERING AREA TO REMAIN OPEN TO SUPPORT DEWATERING UNTIL INSTRUCTED BY ENGINEER.

QUANTITY TABLE

DESCRIPTION	GENERAL FILL (CY)
STEP 2	451,650

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 2 INITIAL FILL PLACEMENT
AND DEWATERING SOUTH PORTION

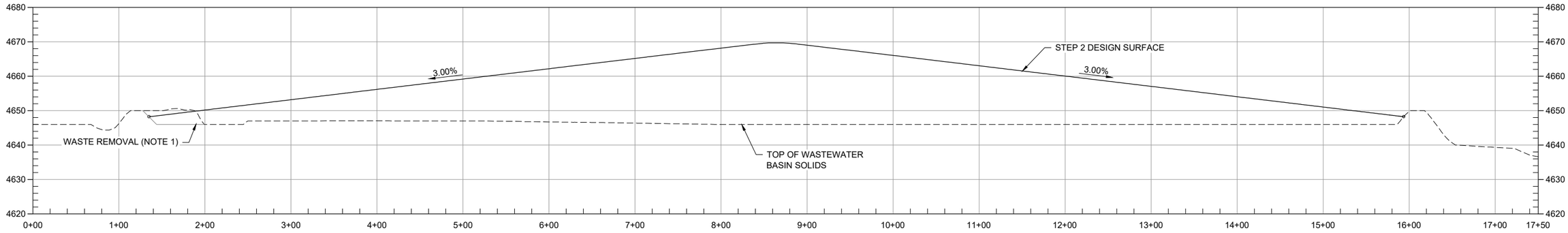
SHEET
C-420

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pw\wrkdir\dms40954\C-420.dwg

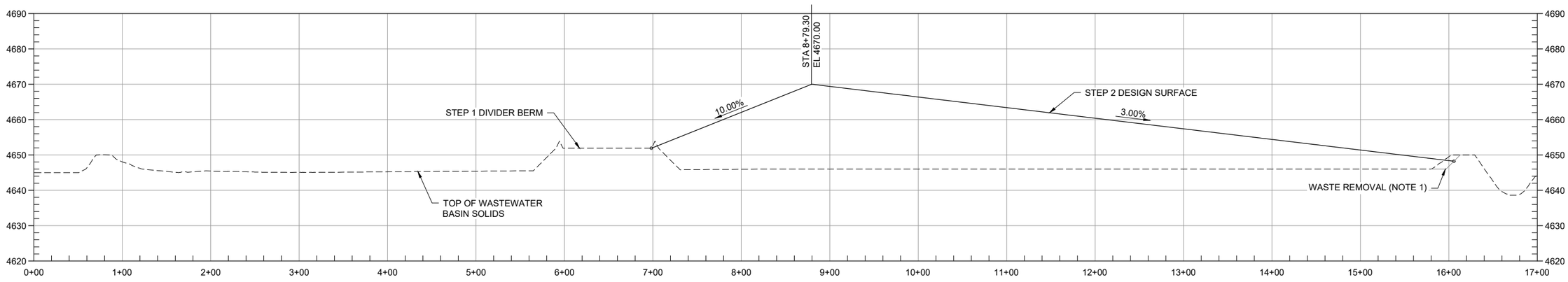
GENERAL SHEET NOTES

1. WASTE SHALL BE REMOVED AROUND EXISTING EMBANKMENT (BY OTHERS) TO FACILITATE COVER SYSTEM CONSTRUCTION.



A C-420 SECTION A

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET



B C-420 SECTION B

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE 1"=100'

WARNING 0 ½ 1 IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN C. FOWLER
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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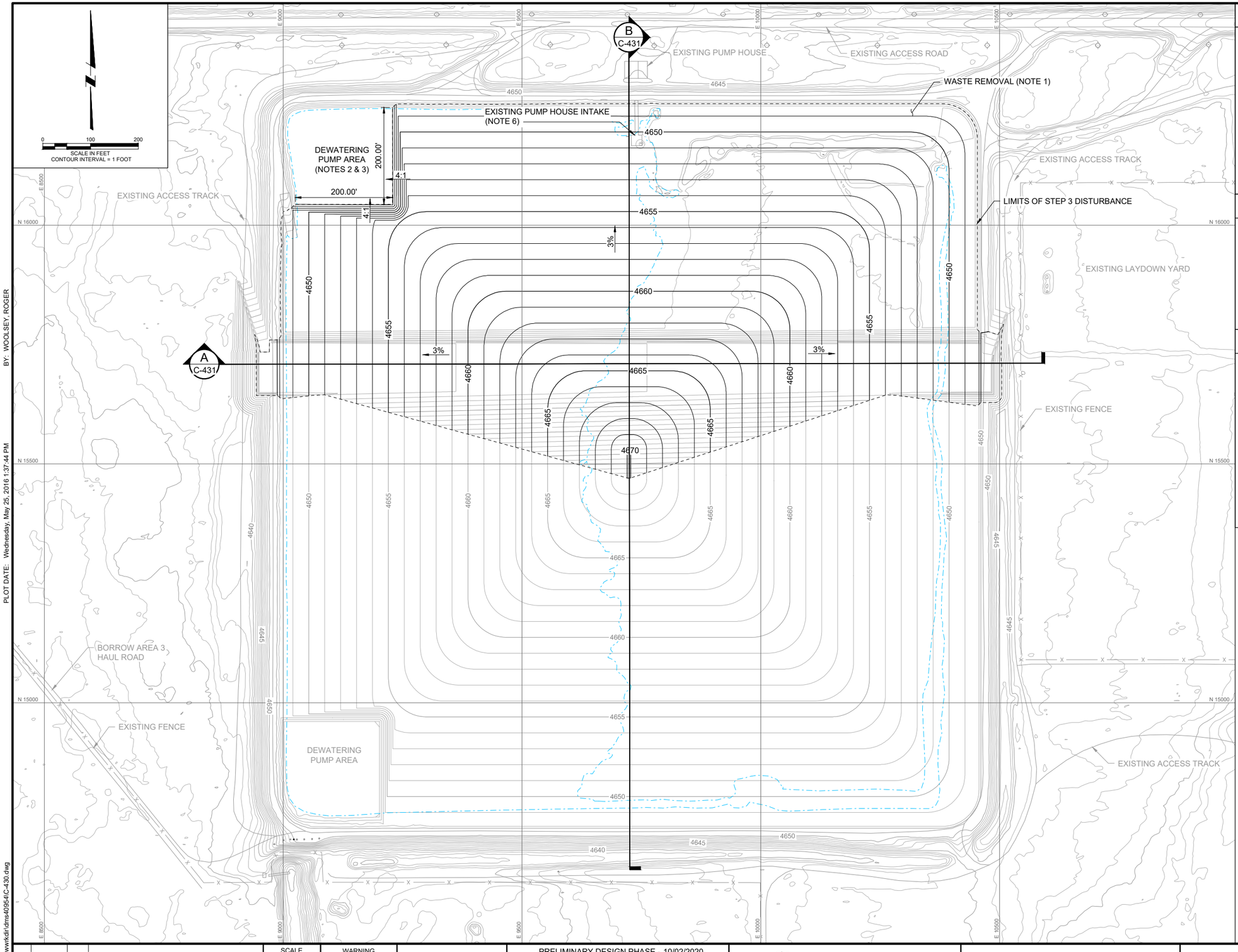
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 2 INITIAL FILL PLACEMENT SECTIONS

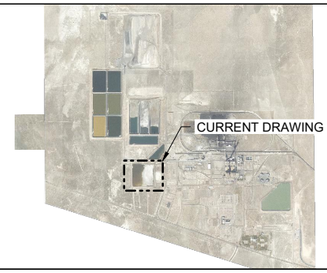
SHEET C-421

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-421.dwg



KEY PLAN



LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- EXISTING FENCE

GENERAL SHEET NOTES

1. WASTE SHALL BE REMOVED AROUND EXISTING EMBANKMENT (BY OTHERS) TO FACILITATE COVER SYSTEM CONSTRUCTION.
2. TEMPORARY SUCTION PUMP AND DISCHARGE LINES SHALL BE PROVIDED BY CONTRACTOR.
3. PUMPED WATER SHALL BE ROUTED TO THE EVAPORATION POND.
4. FILL QUANTITIES ARE BASED ON THE APPROXIMATE TOP OF WASTEWATER BASIN SOLIDS OF 4645 FT AMSL AND REPRESENT IN PLACE QUANTITIES.
5. DEWATERING AREA TO REMAIN OPEN TO SUPPORT DEWATERING UNTIL INSTRUCTED BY ENGINEER.
6. EXISTING PUMP HOUSE INTAKE SHALL BE REMOVED TO THE EXTENT REQUIRED TO SUPPORT FILL PLACEMENT.

QUANTITY TABLE

DESCRIPTION	GENERAL FILL (CY)
STEP 3	254,565

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

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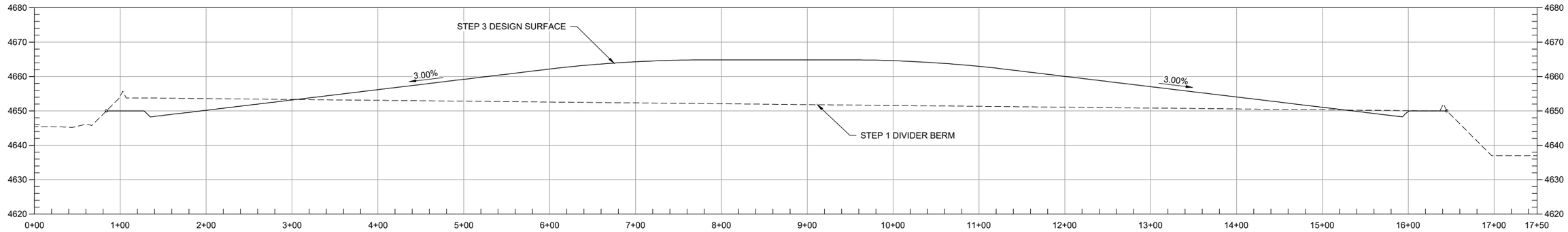
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 3 INITIAL FILL PLACEMENT
AND DEWATERING NORTH PORTION

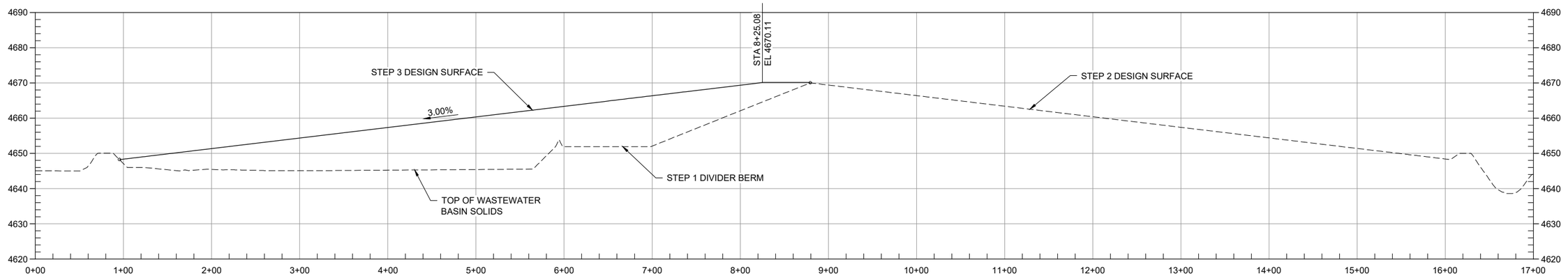
SHEET
C-430
Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-430.dwg



A C-430 SECTION A

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET



B C-430 SECTION B

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE 1"=100'

WARNING
0 ½ 1
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DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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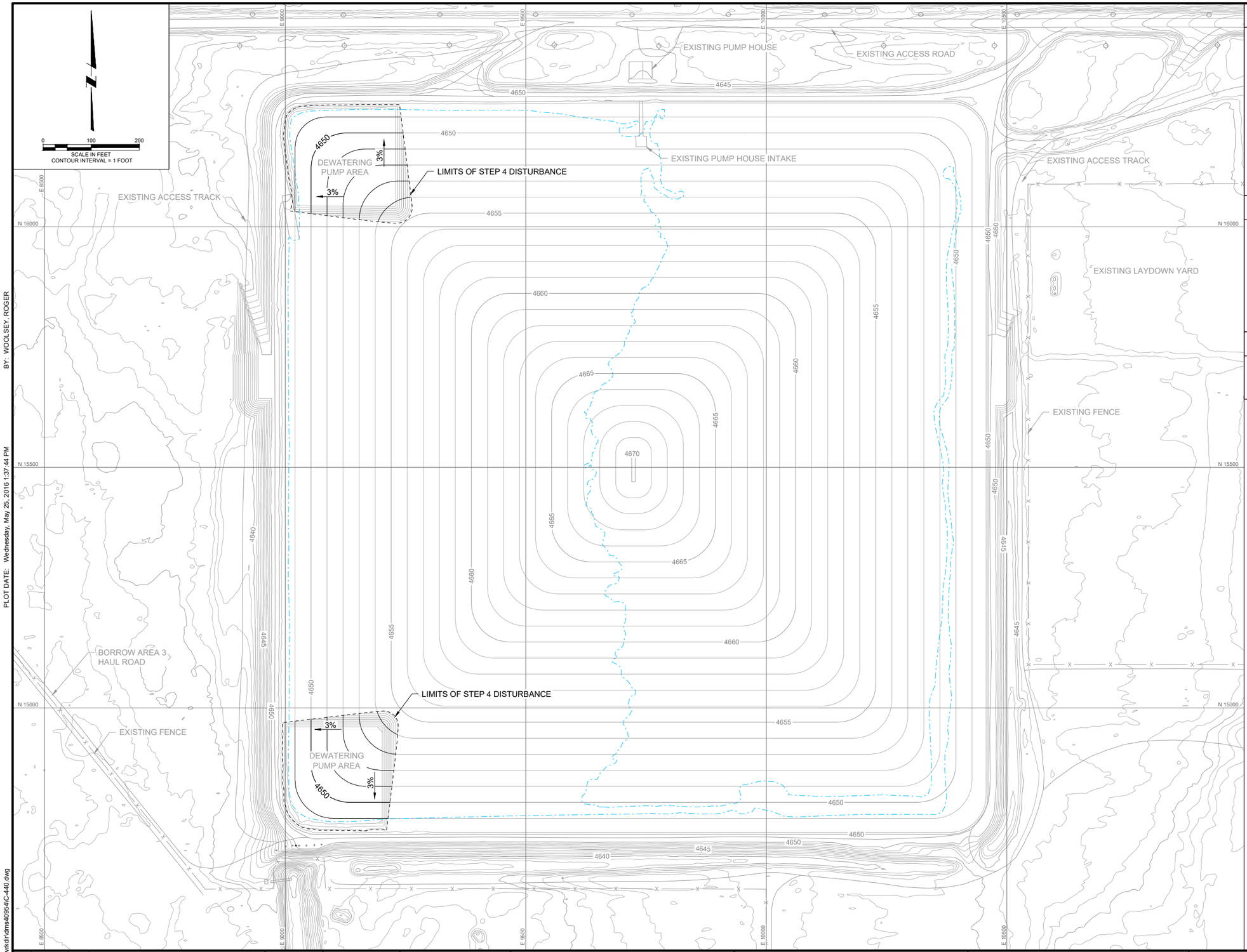
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 3 FILL PLACEMENT SECTIONS

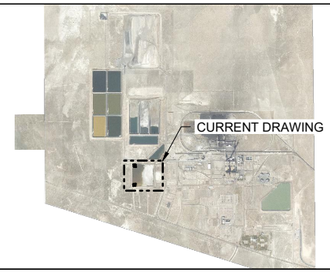
SHEET
C-431

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-431.dwg



KEY PLAN



LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- EXISTING FENCE

GENERAL SHEET NOTES

1. FILL QUANTITIES ARE BASED ON THE APPROXIMATE TOP OF WASTEWATER BASIN SOLIDS OF 4645 FT AMSL AND REPRESENT IN PLACE QUANTITIES.

QUANTITY TABLE

DESCRIPTION	GENERAL FILL (CY)
STEP 4	16,560

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
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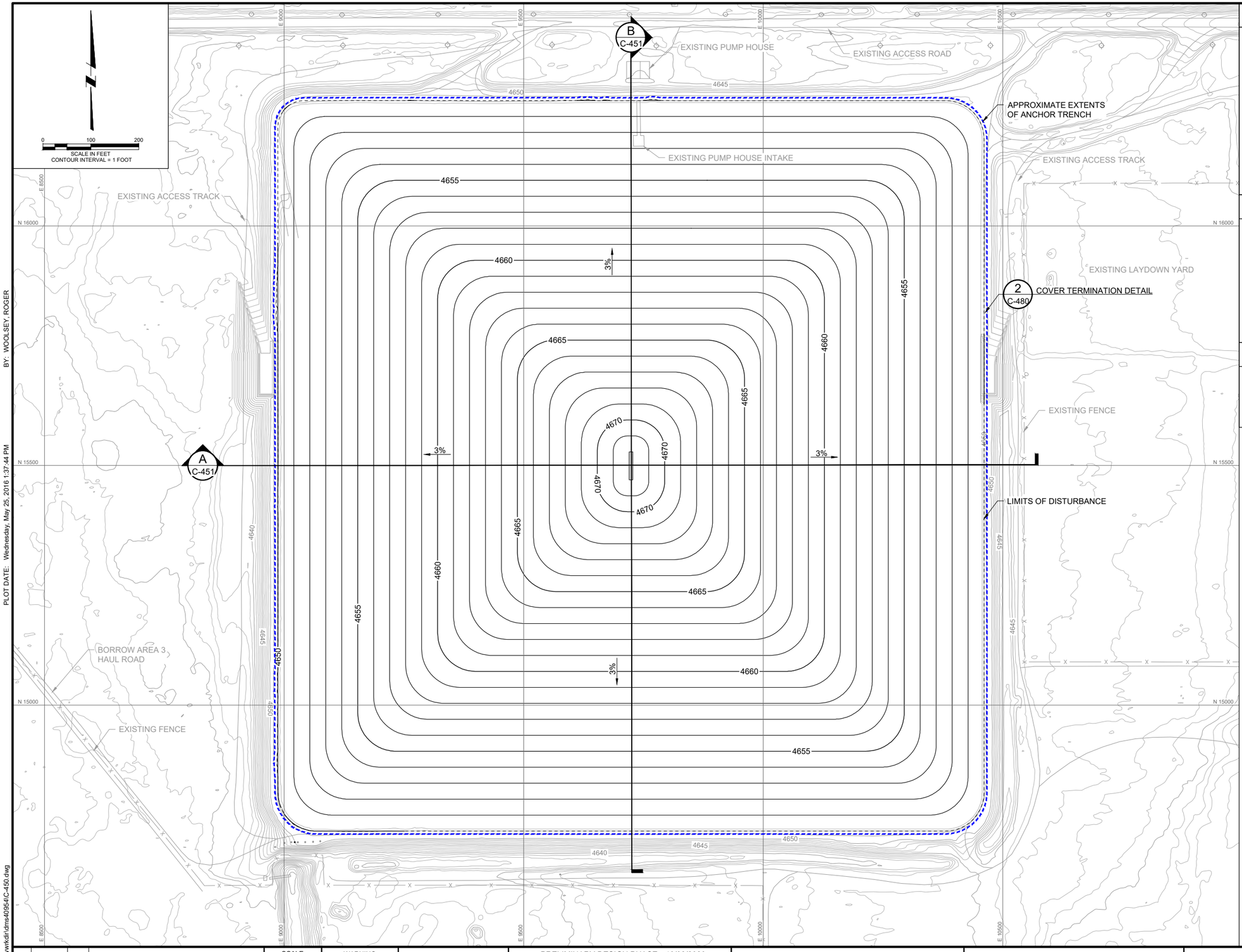
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 4 SUBGRADE PLACEMENT

SHEET
C-440

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pw\wrk\dms40954\C-440.dwg



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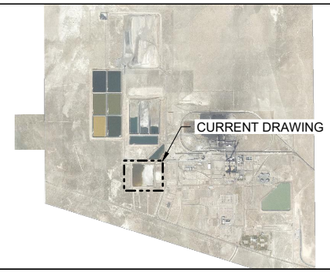

LEGEND

- EXISTING CONTOURS
- DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- APPROXIMATE EXTENTS OF ANCHOR TRENCH
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- EXISTING FENCE

GENERAL SHEET NOTES

1. FILL QUANTITIES ARE BASED ON THE APPROXIMATE TOP OF WASTEWATER BASIN SOLIDS OF 4645 FT AMSL AND REPRESENT IN PLACE QUANTITIES.
2. REFER TO SHEET C-480 FOR TYPICAL SOIL COVER DETAIL.

QUANTITY TABLE

DESCRIPTION	18" GENERAL FILL (CY)	6" TOPSOIL (CY)	LINER (SF)	ANCHOR TRENCH (FT)
STEP 4 AND 5	130,555	41,385	2,243,930	5,912

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

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DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

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INTERMOUNTAIN POWER SERVICE CORP

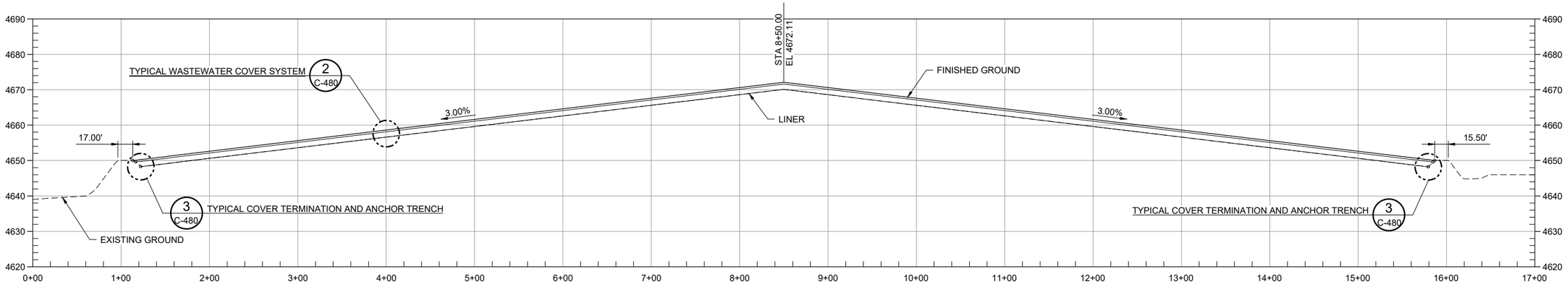
IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 5 FINAL COVER DESIGN

SHEET
C-450

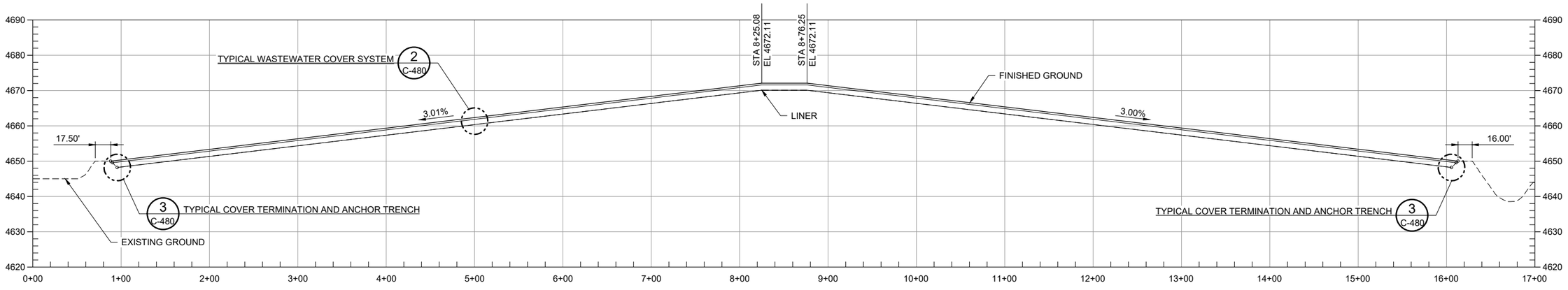
Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM BY: WOOLSEY, ROGER

DWG FILE: C:\pwwrkdir\dms40964\C-450.dwg



A C-450 SECTION A



B C-450 SECTION B

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-451.dwg

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE 1"=100'

WARNING 0 ½ 1 IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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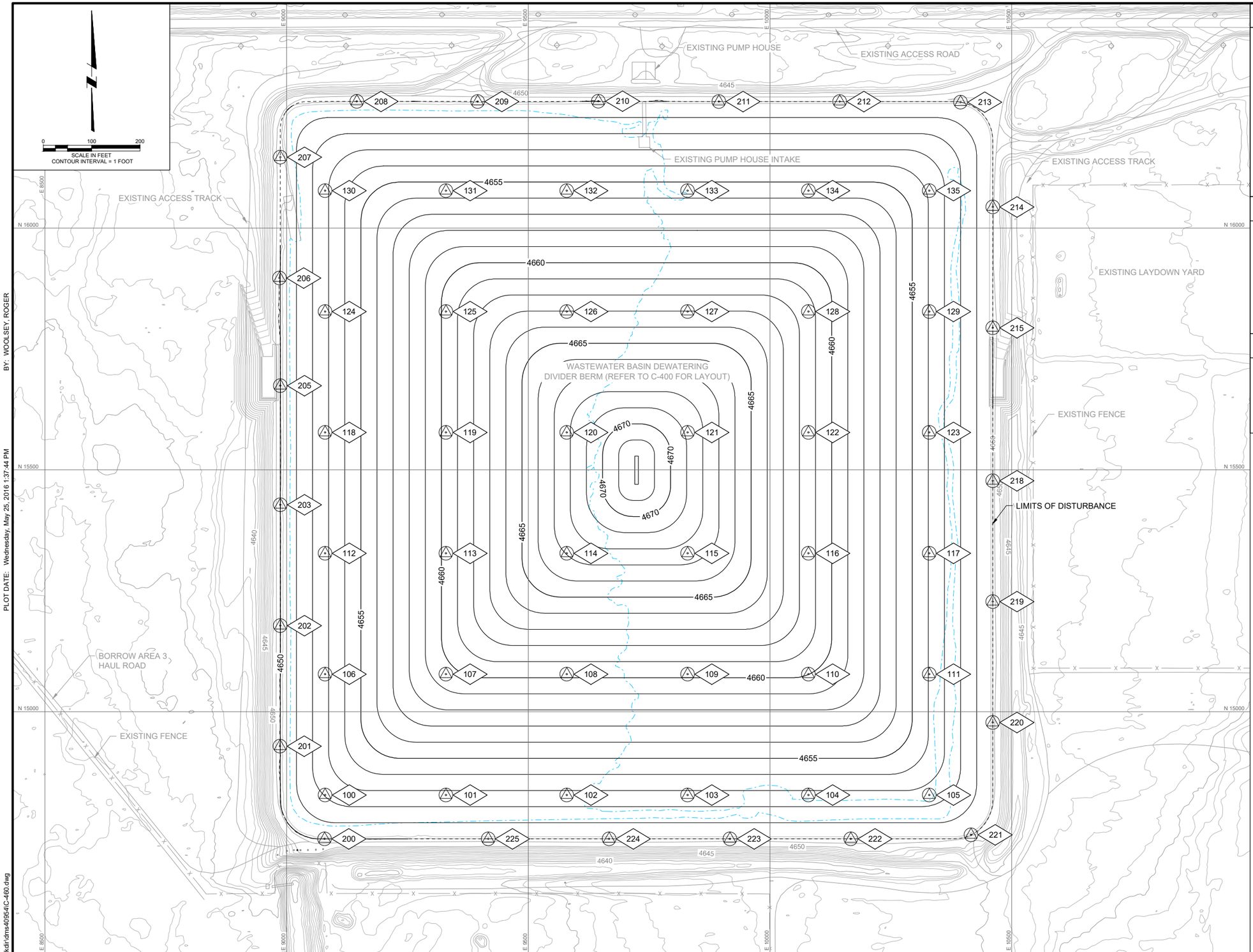
INTERMOUNTAIN POWER SERVICE CORP

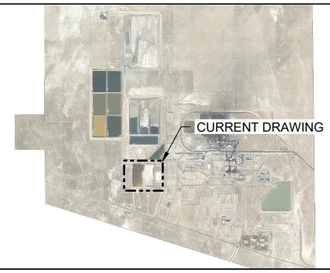
IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 5 FINAL COVER SECTIONS

SHEET
C-451

Job# 233001396



KEY PLAN



LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING WATER LEVEL
- EXISTING POWER POLE
- EXISTING FENCE

GENERAL SHEET NOTES

1. REFER TO SHEET C-461 FOR CONTROL POINT TABLE.
2. CONTROL POINT LOCATIONS ARE PROVIDED FOR INFORMATION ONLY. CONTRACTOR SHALL BE RESPONSIBLE FOR SETTING UP CONTROL POINTS SUFFICIENT TO MEET THE DESIGN LINES AND GRADES.

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-460.dwg

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 5 FINAL COVER CONTROL POINTS

SHEET
C-460

Job# 233001396

GENERAL SHEET NOTES

1. REFER TO SHEET C-460 FOR CONTROL POINT LOCATIONS.
2. CONTROL POINT LOCATIONS ARE PROVIDED FOR INFORMATION ONLY. CONTRACTOR SHALL BE RESPONSIBLE FOR SETTING UP CONTROL POINTS SUFFICIENT TO MEET THE DESIGN LINES AND GRADES.

FINAL COVER POINT TABLE

POINT	NORTHING	EASTING	ELEVATION (FT)
100	14827.47	9079.69	4652.04
101	14827.47	9329.69	4652.72
102	14827.47	9579.69	4652.72
103	14827.47	9829.69	4652.72
104	14827.47	10079.69	4652.72
105	14827.47	10329.69	4652.51
106	15077.47	9079.69	4652.78
107	15077.47	9329.69	4659.54
108	15077.47	9579.69	4660.22
109	15077.47	9829.69	4660.22
110	15077.47	10079.69	4660.01
111	15077.47	10329.69	4653.94
112	15327.47	9079.69	4652.78
113	15327.47	9329.69	4660.28
114	15327.47	9579.69	4667.04
115	15327.47	9829.69	4667.51
116	15327.47	10079.69	4661.44
117	15327.47	10329.69	4653.94
118	15577.47	9079.69	4652.78
119	15577.47	9329.69	4660.28
120	15577.47	9579.69	4667.78
121	15577.47	9829.69	4668.83
122	15577.47	10079.69	4661.44
123	15577.47	10329.69	4653.94
124	15827.47	9079.69	4652.78
125	15827.47	9329.69	4660.28
126	15827.47	9579.69	4663.03
127	15827.47	9829.69	4663.03
128	15827.47	10079.69	4661.33
129	15827.47	10329.69	4653.94
130	16077.47	9079.69	4652.78
131	16077.47	9329.69	4655.53
132	16077.47	9579.69	4655.53
133	16077.47	9829.69	4655.53
134	16077.47	10079.69	4655.53
135	16077.47	10329.69	4653.83

FINAL COVER POINT TABLE

POINT	NORTHING	EASTING	ELEVATION (FT)
200	14736.84	9078.82	4650.00
201	14928.44	8986.51	4649.98
202	15177.80	8987.12	4650.00
203	15427.73	8987.00	4650.00
205	15674.12	8987.12	4650.00
206	15896.62	8985.48	4649.95
207	16146.53	8987.19	4650.00
208	16261.83	9145.41	4650.00
209	16261.78	9395.34	4650.00
210	16262.44	9645.06	4649.98
211	16261.61	9894.29	4650.01
212	16261.61	10144.29	4650.01
213	16260.42	10394.22	4650.01
214	16043.74	10460.73	4650.01
215	15793.74	10460.73	4650.01
218	15477.50	10460.76	4650.01
219	15227.58	10460.79	4650.00
220	14977.58	10460.73	4650.01
221	14745.16	10415.74	4650.01
222	14736.84	10167.19	4650.00
223	14736.84	9917.19	4650.00
224	14736.84	9667.19	4650.00
225	14736.84	9417.19	4650.00

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM BY: WOOLSEY, ROGER

DWG FILE: C:\pwwrkdir\dms40954\C-461.dwg

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: NTS

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

NOT FOR CONSTRUCTION

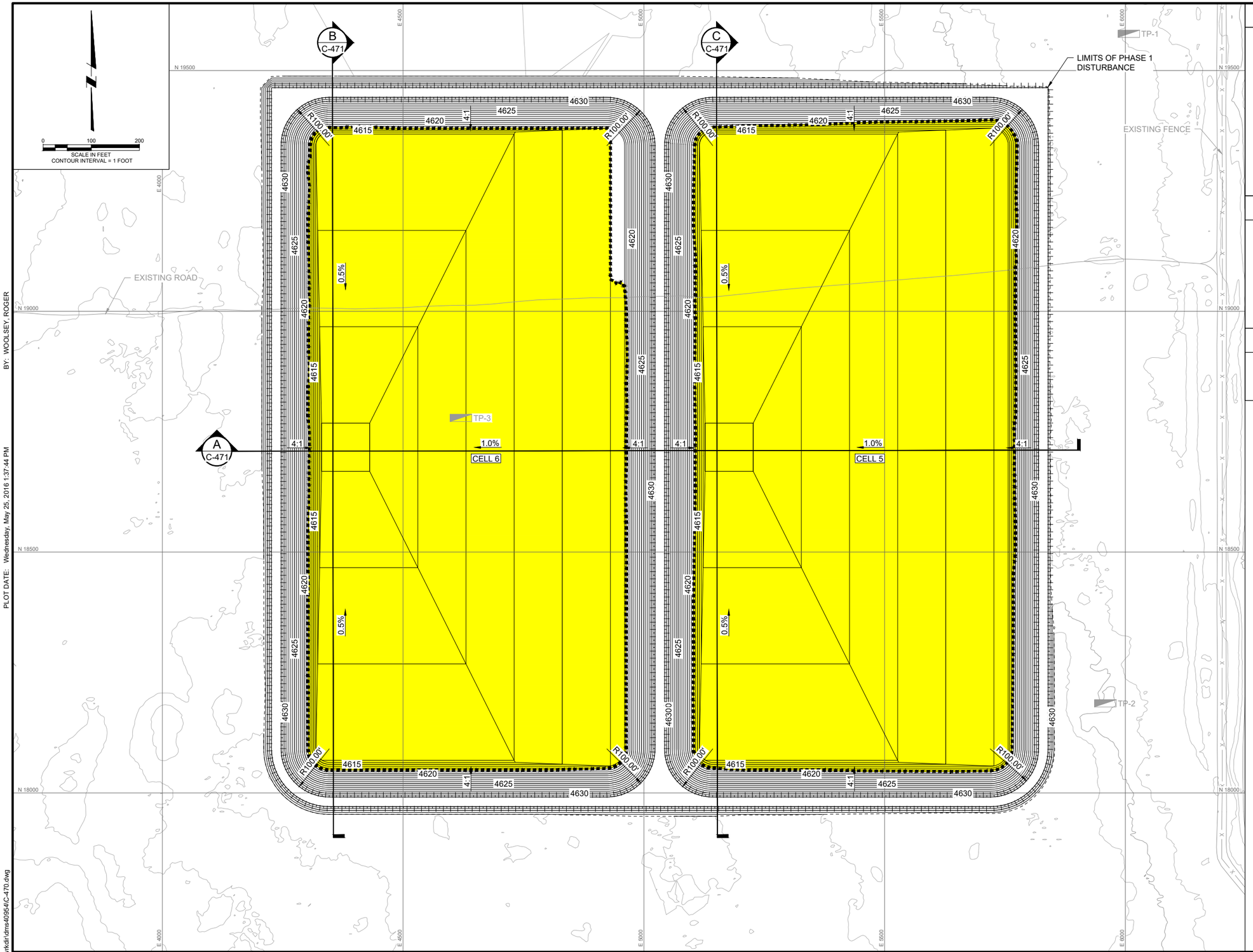
Stantec

INTERMOUNTAIN POWER SERVICE CORP

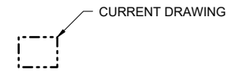
IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
STEP 5 FINAL COVER CONTROL POINTS TABLE

SHEET
C-461

Job# 233001396



KEY PLAN



LEGEND

- 4600 — EXISTING CONTOURS
- 4600 — DESIGN CONTOURS
- LIMITS OF DISTURBANCE
- EXISTING FENCE
- EXSTING TEST PIT
- CLAY EXCAVATION

GENERAL SHEET NOTES

1. REFER TO CLOSURE PLAN FOR DESCRIPTION OF BORROW AREA SOILS.

QUANTITY TABLE			
DESCRIPTION	6" STRIP (CY)	GEN FILL (CY)	CLAY (CY)
EXCAVATION	46,210	668,605	257,175

QUANTITY TABLE	
DESCRIPTION	FILL (CY)
EMBANKMENT FILL	45,450

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE 1"=100'

WARNING 0 ½ 1 IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

NOT FOR CONSTRUCTION

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Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
BORROW SOURCE 3
PHASE 1 EXCAVATION PLAN

SHEET C-470

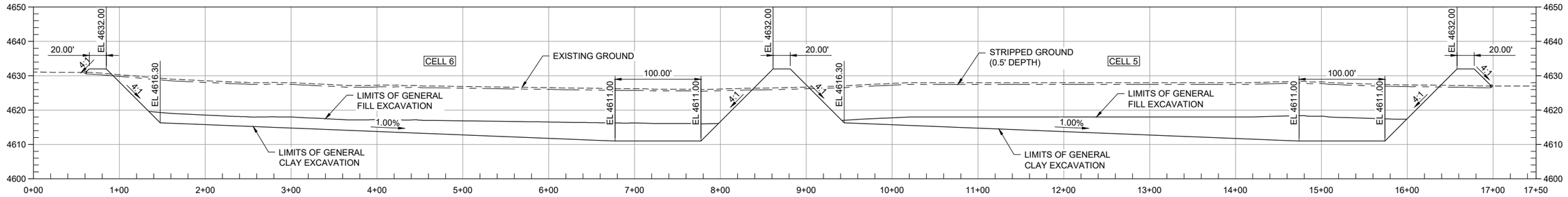
Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM BY: WOOLSEY, ROGER

DWG FILE: C:\pwwtkdir\dms40954\C-470.dwg

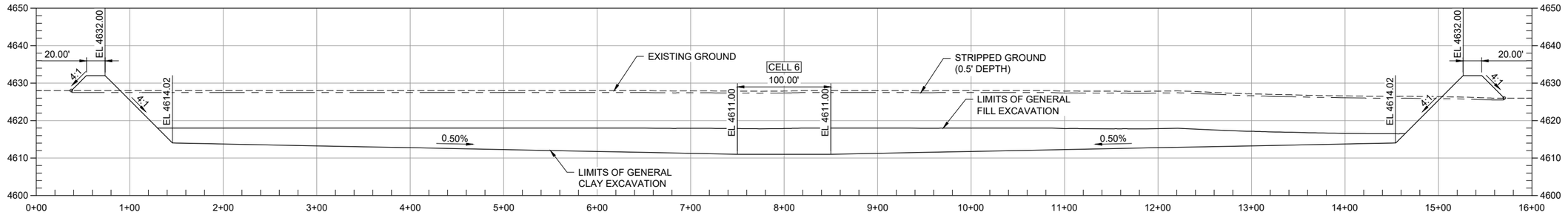
GENERAL SHEET NOTES

1. REFER TO CLOSURE PLAN FOR DESCRIPTION OF BORROW AREA SOILS.



A C-470 SECTION A

HORZ 0 60 120
VERT 0 15 30
SCALE IN FEET



B C-470 SECTION B

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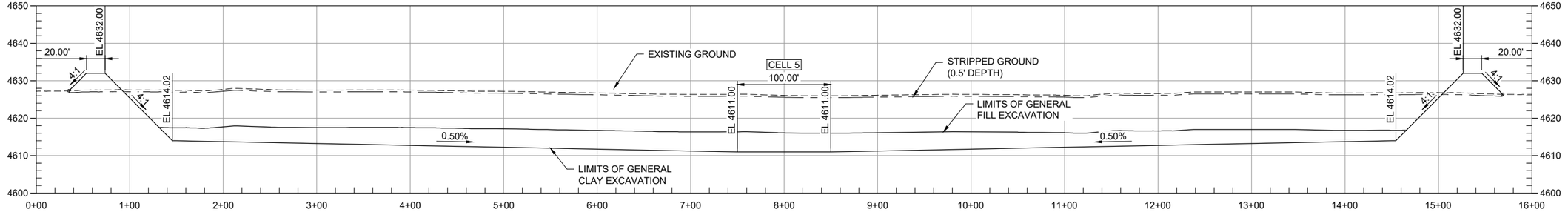

C C-470 SECTION C

HORZ 0 60 120
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SCALE IN FEET

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

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DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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Stantec

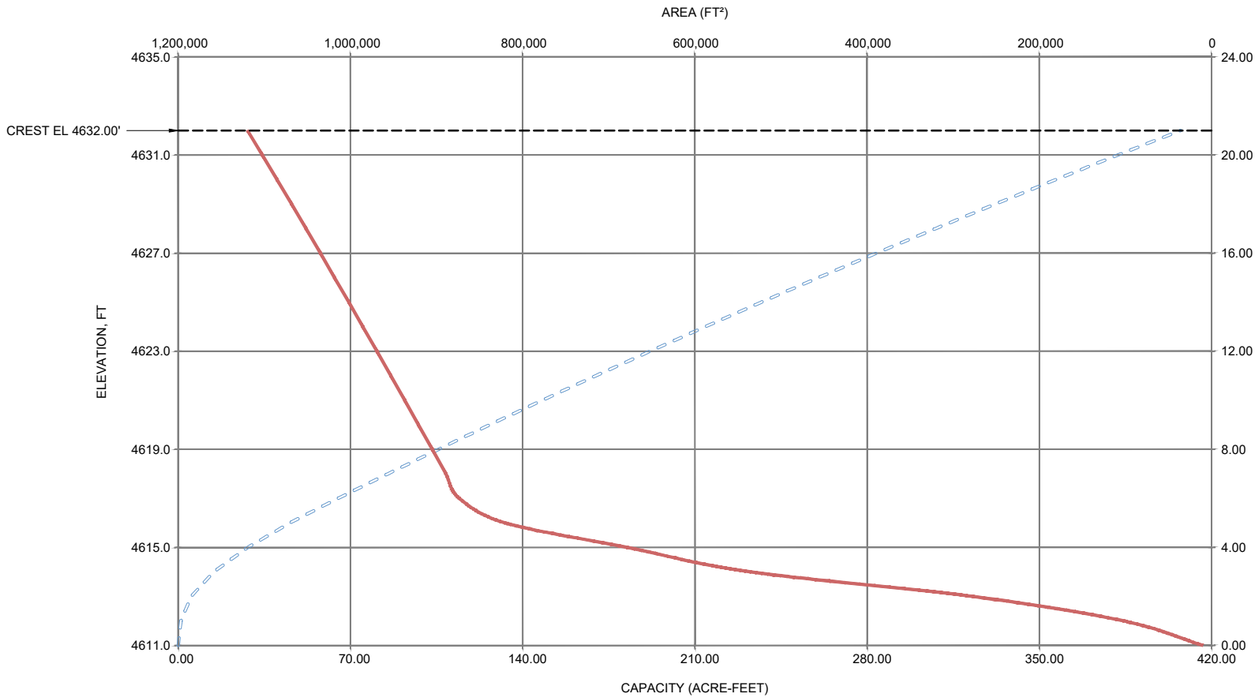
INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
BORROW SOURCE 3
PHASE 1 EXCAVATION SECTIONS

SHEET
C-471

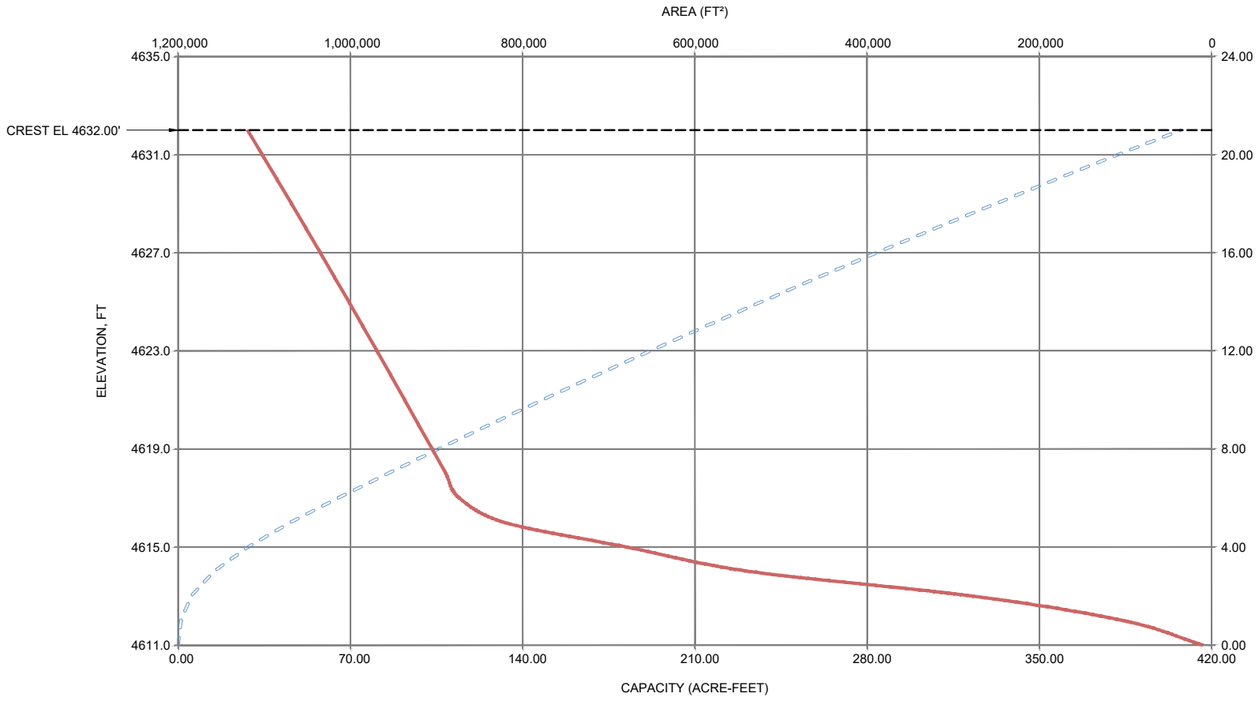
Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-471.dwg



AREA CAPACITY CURVE - EVAP POND 5

AREA CAPACITY TABLE - EVAP POND 5		
ELEVATION (FT)	AREA (FT²)	CAPACITY (ACRE-FT)
4611.00	10,000.00	0.00
4612.00	102,000.00	1.10
4613.00	277,200.00	5.29
4614.00	535,416.14	14.46
4615.00	678,982.01	28.36
4616.00	820,758.87	45.55
4617.00	873,625.86	65.00
4618.00	889,315.00	85.23
4619.00	905,104.51	105.83
4620.00	920,994.37	126.79
4621.00	936,984.59	148.12
4622.00	953,075.17	169.81
4623.00	969,266.11	191.88
4624.00	985,557.40	214.31
4625.00	1,001,949.05	237.13
4626.00	1,018,441.06	260.32
4627.00	1,035,033.43	283.89
4628.00	1,051,726.16	307.84
4629.00	1,068,519.24	332.18
4630.00	1,085,412.68	356.90
4631.00	1,102,406.48	382.01
CREST EL 4632.00' → 4632.00	1,119,500.64	407.52



AREA CAPACITY CURVE - EVAP POND 6

AREA CAPACITY TABLE - EVAP POND 6		
ELEVATION (FT)	AREA (FT²)	CAPACITY (ACRE-FT)
4611.00	10,000.00	0.00
4612.00	102,000.00	1.10
4613.00	277,200.00	5.29
4614.00	535,416.14	14.46
4615.00	678,982.01	28.36
4616.00	820,758.87	45.55
4617.00	873,625.86	65.00
4618.00	889,315.00	85.23
4619.00	905,104.51	105.83
4620.00	920,994.37	126.79
4621.00	936,984.59	148.12
4622.00	953,075.17	169.81
4623.00	969,266.11	191.88
4624.00	985,557.40	214.31
4625.00	1,001,949.05	237.13
4626.00	1,018,441.06	260.32
4627.00	1,035,033.43	283.89
4628.00	1,051,726.16	307.84
4629.00	1,068,519.24	332.18
4630.00	1,085,412.68	356.90
4631.00	1,102,406.48	382.01
CREST EL 4632.00' → 4632.00	1,119,500.64	407.52

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	RNW	ISSUED FOR INTERNAL REVIEW

SCALE: 1"=100'

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN R. WOOLSEY
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

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Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE
BORROW SOURCE 3
PHASE 1 STAGE STORAGE CURVE

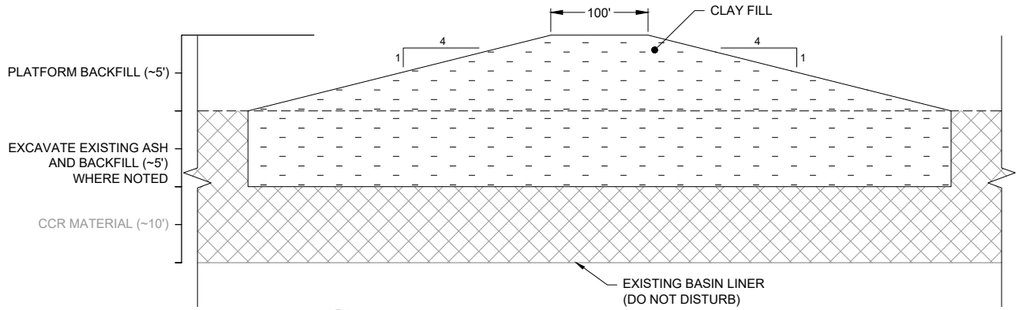
SHEET
C-472

Job# 233001396

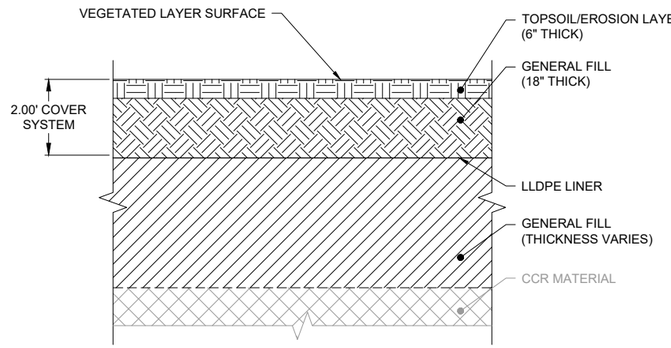
PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\ldms40954\C-472.dwg

GENERAL SHEET NOTES

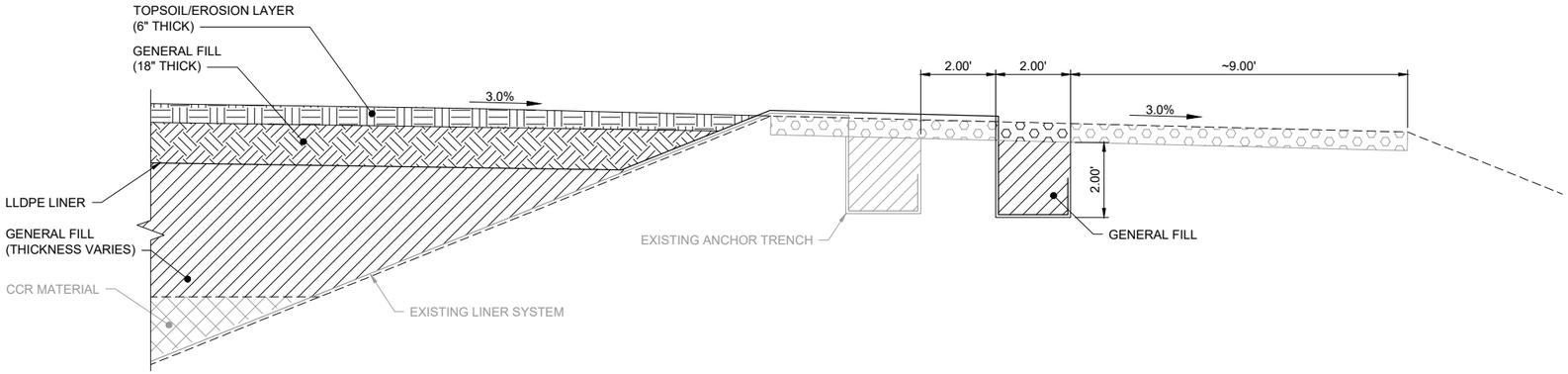
1. REFER SPECIFICATION 31 80 00 FOR LLDPE LINER SPECIFICATION.
2. REFER TO SPECIFICATION 31 00 00 FOR EARTHWORK SPECIFICATION.
3. GEOMEMBRANE SHALL BE PROTECTED IN AREAS OF HEAVY TRAFFIC BY PLACING PROTECTIVE COVER OVER THE GEOMEMBRANE.



1 C-410 WASTEWATER BASIN DEWATERING DIVIDER BERM
NOT TO SCALE



2 C-451 TYPICAL WASTEWATER BASIN COVER SYSTEM
0 2 4
SCALE IN FEET



3 C-451 WASTE WATER BASIN TYPICAL SECTION COVER - CREST TRANSITION
NOT TO SCALE

REV	DATE	BY	DESCRIPTION
B	10/02/2020	RNW	ISSUED FOR CLIENT REVIEW
A	09/11/2020	CF	ISSUED FOR INTERNAL REVIEW

SCALE: AS SHOWN

WARNING: 0 ½ 1 — IF THIS BAR DOES NOT MEASURE 1" THEN DRAWING IS NOT TO SCALE

DESIGNED P. BERNHARD
DRAWN C. FOWLER
CHECKED C. TOMLINSON

PRELIMINARY DESIGN PHASE - 10/02/2020

NOT FOR CONSTRUCTION

Stantec

INTERMOUNTAIN POWER SERVICE CORP

IPSC CCR WASTEWATER BASIN
CIVIL
WASTEWATER BASIN CLOSURE DETAILS

SHEET C-480

Job# 233001396

PLOT DATE: Wednesday, May 25, 2016 1:37:44 PM
BY: WOOLSEY, ROGER
DWG FILE: C:\pwwrkdir\dms40954\C-480.dwg

Appendix B

Construction Specifications

IPP CCR CLOSURE TECHNICAL SPECIFICATIONS

DIVISION 02 - SITEWORK

02222 Earthwork and Grading
02272 Geomembranes
02930 Seeding

SECTION 02222 – EARTHWORK AND GRADING

PART 1 -- GENERAL

1.1 SUMMARY

A. The Contractor shall be responsible for all activities required to ensure that the designated areas are free from objectionable materials, in accordance with the Contract Documents.

B. Contractor shall be responsible for the excavation and grading of the site to configuration in accordance with the details and to the lines and grades indicated by the project drawings.

C. Contractor shall be responsible for construction of the soil covers to the grades and specifications presented herein.

D. The Contractor shall be responsible for development of borrow areas.

1.2 RELATED SPECIFICATION

A. The following specifications contain requirements that relate to this specification:

- 02272 – Geomembranes

1.3 DEFINITIONS

A. Company: Intermountain Power Service Corp.

B. Engineer: Stantec

C. Contractor: The party to whom the Contract for the work described herein has been awarded and any of its authorized representatives.

1.4 CONTRACTOR SUBMITTALS

A. The Contractor shall submit the following documents for Engineer approval and acceptance prior to mobilization:

1. Samples:

a. The Contractor shall submit samples of materials proposed for the Work.

b. Sample sizes shall be determined by the testing laboratory.

PART 2 -- EQUIPMENT AND MATERIALS

2.1 EQUIPMENT

A. Conventional earth-moving equipment shall be used for the material acquisition. All equipment shall be decontaminated prior to arrival at the site, in good working condition, and suitable for its intended use.

2.2 MATERIALS

A. The following materials shall be furnished by the Contractor from designated soil borrow areas or supplied by the Company as specified below.

1. General Fill: General fill material shall be obtained from the identified borrow areas located on the drawings and shall conform to the gradation limits given in Table 1 below, when tested in accordance with ASTM D 422:

Table 1: General Fill Gradation Requirements

U.S. Standard Sieve Size	% Passing	
	Coarse Range	Fine Range
1.5-inch	100	100
¾-inch	90	100
No.4	65	100
No. 40	30	80
No. 200	10	50

2. Compacted Clay Layer, Clay Trench, and Clay Dividing Berm: Compacted Clay Layer, Clay Trench and Clay Dividing Berm material shall be obtained from the identified borrow areas located on the drawings and shall conform to the gradation limits given in Table 2 below, when tested in accordance with ASTM D 422:

Table 2: Compacted Clay Layer, Clay Trench and Clay Dividing Berm Gradation Requirements

U.S. Standard Sieve Size	% Passing	
	Coarse Range	Fine Range
1-inch	100	100
¾-inch	95	100
No.4	90	100
No. 40	80	100
No. 200	60	100

Note that clay clay material can be used for gernal fill if necessary.

3. Topsoil / Erosion Layer: Topsoil / Erosion Layer material shall be 1.5-inch minus material, shall be a blend of 50% clay material and 50% slty sand to promote soil moisture storage and reduce the potential for soil erosion. The Topsoil / Erosion Layer shall conform to the gradation limits given in Table 3 below, when tested in accordance with ASTM D 422.

Table 3: Topsoil / Erosion Layer Material Gradation Requirements

`	% Passing	
	Coarse Range	Fine Range
1.5-inch	100	100
¾-inch	90	100
No. 4	65	100
No. 40	50	95
No. 200	30	75

PART 3 -- EXECUTION

3.1 EXCAVATION

A. General

1. Excavation is unclassified and includes excavation to required grade, or subgrade elevations, regardless of the character of materials or obstruction encountered.

2. Tolerances for all excavated surfaces shall be within ±0.1 foot of the elevation as specified in the design drawings.

3. Excavations shall be sloped or otherwise supported in a safe manner in accordance with applicable state safety requirements and the requirements of OSHA Safety and Health Standards for Construction (29CFR1926).

4. The Contractor shall provide quantity surveys where so required to verify quantities for Unit Price Contracts.

5. Survey shall be performed prior to beginning Work and upon completion by a surveyor licensed in the State of Utah.

6. If stockpiles will be used, the material shall be transported and stockpiled in an approved stockpiling area.

B. Disposal Of Excess Excavated Material

1. The Contractor shall be responsible for the removal and stockpiling of any excess excavated material according to Section 01552 – Staging and Stockpile Areas.

2. Material shall be disposed of at an approved on-Site disposal area.

3.2 FILL PLACEMENT AND COMPACTION

A. Material Placement

1. Material shall be placed and spread evenly in approximately horizontal layers.

2. Lift thicknesses are specified by material types in the following sections.

3. Unless otherwise approved by the Engineer, loose lift thickness shall not exceed 6 inches, prior to compaction by hand operated compactors.

B. General Fill:

1. General Fill shall be spread in 18-inch loose lifts using equipment meeting the ground pressure requirements described in Section 02272 – Geomembrane to prevent damage to the geomembrane. The equipment shall have GPS elevation grade control capability.

2. Following placement and grading of each lift, the surface shall be compacted with a number of passes by equipment that is capale of achieving the required degree of compaction stated in Table 4.

3. Following placement and grading of the general fill for the liner protection layer, the surface shall be compacted with a number of passes (tracked) by the low-groud-pressure (LGP) dozer. The Contractor shall determine the appropriate number of passes to achieve the required degree of compaction stated in Table 4.

4. Moisture contents of the general fill during placement shall comply with Table 4.

C. Compacted Clay Layer, Clay Trench and Clay Divider Berm:

1. Compacted Clay Layer shall be spread in 8-inch loose lifts. The equipment used to spread lifts shall have GPS elevation grade control capability.

2. Clay Trench and Clay Divider Berm shall be spread in 12-inch loose lifts. The equipment used to spread lifts shall have GPS elevation grade control capability.

3. Following placement and grading of each lift, the surface shall be compacted with a number of passes by equipment that is capale of achieving the required degree of compaction stated in Table 4. The Contractor shall determine the appropriate number of passes.

4. Moisture contents of the Compacted Clay Layer, Clay Trench and Clay Divider Berm during placement shall comply with Table 4.

5. Where clay is to be used as General Fill the contractor shall place, spread, and compact the layer in accordance with Section 3.2.B

D. Topsoil / Erosion Layer:

1. Topsoil Layer shall be spread in one loose lift using equipment meeting the ground pressure requirements described in Section 02272 – Geomembrane to prevent damage to the geomembrane, graded to achieve final design grades, and compacted to meet the requirements of Table 4, by tracking to achieve the final thickness.

2. The surface of the layer shall be tracked into place to maintain the surface of the material, in the event of heavy rain, prior to vegetation.

E. Compaction Requirements:

1. Compaction equipment shall be of the appropriate type and weight for the fill materials being placed in order to achieve the compaction requirements of this Specification and meet the ground pressure requirements described in Section 02272 – Geomembrane where applicable.

2. The Contractor shall submit compaction procedures to the Engineer as part of the Construction Plan submitted. Procedures shall include details of the equipment proposed for use and the number of passes required. The Contractor shall state in the procedures, the steps that will be taken to control moisture content of the fill materials. Approval of the compaction procedures shall be given by the Engineer prior to Contractor undertaking any compaction work.

3. Coverages of Compaction Equipment: Coverages of the compaction equipment shall be carried out so that the compactive effort is uniformly distributed in a systematic manner over the entire lift. Compaction of individual lanes of a lift shall be completed before beginning compaction of adjacent portions of the lift. Individual lanes shall be overlapped by at least 1 ft.

4. In locations where compaction by normal mechanical equipment is not possible and compaction can only be completed by hand tamping, fill shall be moistened, placed and compacted with the aid of pneumatic or hand tampers. Pneumatic and hand tampers shall provide a minimum of 9 psi compactive force.

5. Compaction shall meet the requirements given in Table 4 below in accordance with:

   ASTM D698 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (400 ft-lbf/ft^3) where the material is graded such that 10 percent or more passes a No. 4 sieve.

Table 4: Compaction Requirements for Fill Materials

Location or Use of Fill or Backfill	Percentage of Maximum Dry Density	Percentage of Optimum Moisture
General Fill	90% (±3% of MDD)	±2%
General Fill (Liner Protective Layer)	90% (±3% of MDD)	±2%
Compacted Clay Layer (CB Landfill)	95% (minimum)	±2%
Clay Trench (Wastewater Basin)	90% (±3% of MDD)	NA
Clay Divider Berm (Wastewater Basin)	90% (minimum)	±2%
Erosion Protection Layer (topsoil)	85% (+5%)	±2%

F. Moisture Content

1. For General Fill, Compacted Clay Layer, Clay Divider Berm and Topsoil, the moisture contents of materials to be placed and compacted or scarified and compacted shall be within +2.0 and -2.0 percent of the Optimum Moisture Content (OMC) as

determined by ASTM D 698 – Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12 400 ft-lbf/ft^3).

2. The moisture content of materials shall be uniform throughout each layer of material placed prior to and during compaction.

3. Perform wetting and drying operations as necessary in order to achieve the required moisture contents prior to compaction.

4. Materials too dry for compaction shall be pre-wetted in the borrow areas. Supplemental water, if required, shall be added to the material at the placement area prior to compaction; by uniform sprinkling, followed by uniform mixing, prior to compaction.

5. Materials too wet for compaction shall be dried to the proper moisture content before compaction. Mixing of wet materials with drier materials may also be performed to achieve the appropriate moisture content, as approved by the Engineer.

6. If the moisture content of fill material placed into the work falls outside the required limits, the Contractor shall condition the material to bring it to within the required limits. If the material cannot be brought readily to the specified moisture content, the Contractor shall remove the material from the work.

3.3 MATERIALS TESTING

A. Samples:

1. Soils testing of samples submitted by the Contractor will be performed by a testing laboratory of the Contractor's choice and at the Contractor’s expense.

2. The Engineer may direct the Contractor to supply samples for testing of any material used in the Work.

3. Particle-size analyses of soils and aggregates will be performed using ASTM D 422 - Standard Test Method for Particle-size Analysis of Soils.

4. References in this Section to soil classification types and standards shall have the meanings and definitions indicated in ASTM D 2487.

5. The Contractor shall be bound by applicable provisions of ASTM D 2487 in the interpretation of soil classifications.

B. Field and Laboratory Testing:

1. Field soils testing will be performed by a testing laboratory of the Contractor's choice at the Contractor's expense at the frequency given in Table 5 below.

Table 5: Minimum Required QC Field and Laboratory Testing Methods and Frequencies

Material	Test Name	Testing Method	Minimum QC Testing Frequency
General Fill / Liner Protective Layer	In-Place Moisture and Density – Nuclear Moisture Density Gauge	ASTM D6938	1 per 10,000 CY and per each material source or processing method
	Standard Proctor	ASTM D698	1 per 20,000 CY and per each material source or processing method
	Atterberg Limits	ASTM D4318	1 per Proctor test
	Classification of Soils	ASTM D2487	1 per Proctor test
	Grain-size Distribution	ASTM D6913	1 per Proctor test
Compacted Clay Layer (CB Landfill)	In-Place Moisture and Density – Nuclear Moisture Density Gauge	ASTM D6938	1 per 5,000 CY and per each material source or processing method
	Standard Proctor	ASTM D698	1 per10,000 CY and per each material source or processing method
	Atterberg Limits	ASTM D4318	1 per Proctor test
	Classification of Soils	ASTM D2487	1 per Proctor test
	Grain-size Distribution	ASTM D6913	1 per Proctor test
Clay Ternch / Clay Dividing Berm	In-Place Moisture and Density – Nuclear Moisture Density Gauge	ASTM D6938	1 per 5,000 CY and per each material source or processing method
	Standard Proctor	ASTM D698	1 per 10,000 CY and per each material source or processing method
	Atterberg Limits	ASTM D4318	1 per Proctor test
	Classification of Soils	ASTM D2487	1 per Proctor test
	Grain-size Distribution	ASTM D6913	1 per Proctor test
Topsoil/Erosion Layer	In-Place Moisture and Density – Nuclear Moisture Density Gauge	ASTM D6938	1 per 2,000 CY and per each material source or processing method
	Standard Proctor	ASTM D698	1 per 5,000CY and per each material source or processing method
	Atterberg Limits	ASTM D4318	1 per Proctor test
	Classification of Soils	ASTM D2487	1 per Proctor test
	Grain-size Distribution	ASTM D6913	1 per Proctor test

Notes:
1. The Engineer may revise the listed frequencies and test methods during the work.
2. Standard Proctor testing shall be performed at the frequencies listed in the table and as needed to obtain Proctor values representative of the placed material.

C. Contractor's Responsibilities:

1. Re-working to Attain Specified Limits: When the test results indicate that compaction, water content, or relative compaction is not in conformance with specified limits, the Contractor shall make immediate adjustments in procedures as necessary to conform to the specified limits. Re-working to attain the specified limits may include removal, rehandling reconditioning, re-rolling, or combinations of these procedures. The Contractor shall perform all re-work required to achieve the specified compaction water content and relative compaction at no cost to the Company.

2. Confirmation of In-Situ Material Properties: The Contractor shall independently confirm the geotechnical properties of the proposed Cover Soil material and determine the appropriate moisture conditioning and compaction methods to ensure that cover material meets the project specifications and are constructed to the design lines and grades as provided in the design drawings. Claims arising from material shrinkage and/or swelling will not be entertained.

- END OF SECTION -

SECTION 02272 –GEOMEMBRANES

PART 1 -- GENERAL

1.1 SUMMARY

A. The CONTRACTOR shall supply all labor, equipment, materials, and appurtenances for the complete installation of geomembranes as per contract documents.

B. Sufficient geomembrane material shall be furnished to cover all lined areas, including seam overlaps and anchor trenches. One percent shall be added to the length of each panel to allow for shrink and wrinkles. The geomembrane shall be installed in a relaxed condition and shall be free of tension or stress upon completion of the installation.

1.2 SUBMITTALS

A. Prior to installation of geomembrane material, the CONTRACTOR shall submit the following for the ENGINEER's approval:

1. Resin Data, including a certification stating that the resin meets the specification requirements (see Paragraph 2.3.C).

2. Statement certifying that geomembrane materials have been tested and inspected in accordance with Paragraph 1.5.

3. Statement certifying no recycled polymer and no more than 10% rework of the same type of material is added to the resin (product run may be recycled).

4. Specification sheet stating that the geomembrane meets the specification requirements (see Paragraph 2.3.E)

5. Installation layout drawings showing the proposed panel layout to cover the lined area shown, with proposed size, number, position, and sequence of placing all sheets and indicating the location and direction of all field joints and penetrations. Installation layout drawings shall also show complete details and/or methods for anchoring, field joints, seals at existing structures, etc.

6. Four 8-inch x 10-inch samples of the material proposed for the lining

7. A Statement of Qualifications for the geomembrane manufacturer and installation contractor with sufficient detail to satisfy the experience requirements of Paragraph 1.3.

8. Installation Contractor's Quality Control Plan.

B. Placement of geomembrane material shall not commence until the submittals required in Paragraph 1.2 A have been approved by the ENGINEER.

C. Upon completion of geomembrane installation, the CONTRACTOR shall submit the following:

1. Certificate stating the geomembrane has been installed in accordance with the Contract Documents.

2. Material and installation warranties

3. As-built drawings showing actual geomembrane placement and seams including complete details.

1.3 QUALIFICATIONS

A. **Qualifications of Manufacturer:** The manufacturer shall have at least five years continuous experience in manufacturing polyethylene geomembrane and/or experience totaling not less than 5 million square feet of manufactured polyethylene geomembrane.

1. The following manufacturers are approved by the COMPANY:

a. Agru America

b. Solmax

B. **Qualifications of Installation Contractor:** The installation contractor shall be the manufacturer, or shall be trained to install the manufacturer's material, and shall have experience of not less than 3 projects and not less than 1,000,000 square feet of successfully installed polyethylene geomembrane.

1. Field Installation Supervisor: Installation shall be performed under the constant direction of a Field Installation Supervisor who shall remain on site and be responsible, throughout the geomembrane installation, for layout, seaming, testing, repairs, and all other activities by the Installer. The Field Installation Supervisor shall have installed or supervised the installation of not less than 1,000,000 square feet of polyethylene geomembrane.

2. Master Seamer: Seaming shall be performed under the direction of a Master Seamer (who may also be the Field Installation Supervisor) who has seamed not less than 1,000,000 square feet of polyethylene geomembrane, using the same type of seaming apparatus specified for this project. The Field Installation Supervisor and/or Master Seamer shall be present whenever seaming is performed.

1.4 REFERENCE SPECIFICATIONS, CODES AND STANDARDS

ASTM D792	Test Method for Density and Specific Gravity (Relative Density) of Plastics by Displacement
ASTM D1004	Test Method for Initial Tear Resistance of Plastic Film and Sheeting
ASTM D1238	Standard Test Method for Flow Rates of Thermoplastics by Extrusion Plastometer
ASTM D1505	Test Method for Density of Plastics by the Density-Gradient Technique
ASTM D1603	Test Method for Carbon Black in Olefin Plastics
ASTM D3895	Standard Test Method for Oxidative-Induction Time of Polyolefins by Differential Scanning Calorimetry
ASTM D4218	Standard Test Method for Determination of Carbon Black in Polyethylene Compounds
ASTM D4833	Standard Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products

ASTM D5199	Standard Test Method for Measuring Nominal Thickness of Geotextiles and Geomembranes
ASTM D5397	Standard Test Method for Evaluation of Stress Crack Resistance of Polyolefin Geomembranes Using Notched Constant Tensile Load Test
ASTM D5596	Standard Test Method for Microscopic Evaluation of the Dispersion of Carbon Black in Polyolefin Geosynthetics
ASTM D5994	Standard Test Method for Measuring Core Thickness of Textured Geomembranes
ASTM D6392	Standard Test Method for Determining the Integrity of Nonreinforced Geomembrane Seams Produced Using Thermo-Fusion Methods
ASTM D6693	Standard Test Method for Determining Tensile Properties of Nonreinforced Polyethylene and Nonreinforced Flexible Polypropylene Geomembranes
ASTM D7240	Standard Practice for Leak Location using Geomembranes with an Insulating Layer in Intimate Contact with a Conductive Layer via Electrical Capacitance Technique (Conductive Geomembrane Spark Test)
GRI GM 13	Test Methods, Test Properties, and Testing Frequency for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes
GRI GM 14	Standard Guide for Selecting Variable Intervals for Taking Geomembrane Destructive Seam Samples Using the Method of Attributes
GRI GM 17	Test Methods, Test Properties and Testing Frequency for Linear Low Density Polyethylene (LLDPE) Smooth and Textured Geomembranes

1.5 QUALITY CONTROL

A. All WORK shall be constructed, monitored and tested in accordance with the requirements of the Installation Contractor's Quality Control Plan (CQP), which shall be submitted in accordance with Paragraph 1.2 A.

B. The CONTRACTOR shall be aware of all activities outlines in the CQP, and the CONTRACTOR shall account for these activities in the construction schedule. No additional costs to the COMPANY shall be allowed by the CONTRACTOR as a result of the performance of the CQP activities.

1.6 QUALITY ASSURANCE

A. The COMPANY shall conduct quality assurance monitoring and testing of the geomembrane installation under the direction of the ENGINEER. This testing is defined in Part 3 of the Specification and include, but are not limited to, trial welds (Section 3.2.F.5) and seam testing (Section 3.3).

1.7 WARRANTY

A. The CONTRACTOR shall procure and provide copies of the manufacturer's warranty for the geomembrane system and all appurtenances. The warranty shall cover materials for a period of 5 years prorated and workmanship for a period of 1 year from the date of the COMPANY's acceptance of the project. The warranty shall not be prorated for workmanship, but shall be a full replacement value warranty. Should defects or premature loss of use within the scope of the above warranty occur, repair and/or replacement of damaged material shall be performed by the CONTRACTOR at no cost to the COMPANY.

PART 2 -- PRODUCTS

2.1 SCHEDULE OF GEOMEMBRANES

TABLE 1 – SCHEDULE OF GEOMEMBRANES

Application	Geomembrane
Bottom Ash Basin Cover Geomembrane	60-mil HDPE, Textured (Single Side)
Wastewater Basin Cover Geomembrane	60-mil LLDPE, Textured (Single Side)

2.2 APPROVED GEOMEMBRANE PRODUCTS

A. 60-mil HDPE, Textured (Single Side)

1. Solmax HDPE Single Textured

2. Agru America HDPE MicroSpike Single Sided

B. 60-mil LLDPE, Textured (Single Side)

1. Solmax LLDPE Single Textured

2. Agru America LLDPE MicroSpike Single Sided

2.3 "OR EQUAL" PRODUCTS

A. CONTRACTOR shall provide the COMPANY approved geomembrane products listed in Paragraph 2.2, or provide "or equal" products that meet the requirement indicated below.

B. **Materials:** The material shall be black, coextruded high-density polyethylene (HDPE) geomembrane or black, coextruded linear low-density polyethylene (LLDPE) geomembrane as listed below and as shown on the Contract Drawings.

C. The geomembrane shall be manufactured from new, first quality resin produced in the United States and shall be compounded and manufactured specifically for producing geomembrane. Natural resin (without carbon black) shall meet requirements listed in Table 2:

TABLE 2 – RESIN PROPERTIES

Property	Test Method	HDPE Value	LLDPE Value
Density (g/cm3)	ASTM D 792 / ASTM D 1505	≥0.932	≤0.926
Melt Flow Index (g/10 min)	ASTM D 1238 (190/2.16)	≤1.0	≤1.0

Reprocessed materials shall not be acceptable. No post-consumer resin of any type shall be added to the formulation.

D. **Fabrication:** The geomembrane shall have a minimum 20-foot seamless width. The geomembrane shall be supplied in rolls with labels identifying the thickness of material, the length and width of the roll, the lot and roll numbers, and the name of the manufacturer.

E. Properties:

1. The geomembrane shall not exceed a combined maximum total of 1 percent by weight of additives other than carbon black.

2. The geomembrane shall be free of holes, pinholes, bubbles, blisters, excessive contamination by foreign matter, and nicks and cuts on roll edges.

3. The finished product shall be uniform in color, thickness, and surface texture and shall meet the **minimum** average specifications listed in Table 3 and as stipulated in GRI Test Method GM13 and GM17 for HDPE and LLDPE liners, respectively.

F. Manufacturer Quality Control

1. All resins and additives used in the fabrication of the geomembrane shall be sampled, tested, and approved by the MANUFACTURER before being eligible for use. Sampling and testing of the resins and additives shall be performed in accordance with the Manufacturer's Quality Control program.

2. All roll goods shall be inspected for defects and impurities. Geomembrane thickness shall be measured for each roll.

3. All geomembrane sheets produced at the factory shall be inspected prior to shipment for compliance with the physical property requirements listed in Paragraph 2.3.E and be tested by an acceptable method of inspecting for pinholes. If pinholes are located, identified and indicated during manufacturing, these pinholes may be corrected during installation.

4. The geomembrane shall be tested by the MANUFACTURER for the listed properties provided in the tables in Part 2. A log shall be maintained showing the testing date, time and results. Any rolls not meeting the visual inspection or requirements of the specification shall be rejected.

5. Certification that the material has been inspected, tested, and meets all requirements shall be submitted to the ENGINEER. Test results shall be made available to the ENGINEER upon request.

TABLE 3 – GEOMEMBRANE PROPERTIES

Tested Property	Test Method	Frequency	Textured HDPE	Textured LLDPE
Thickness, (minimum average) mil; Lowest individual reading (-10%);	ASTM D 5199 (Sm.) / ASTM D 5994 (Tx.)	every roll	60 54	60 54
Density, g/cm^3	ASTM D 792 / ASTM D 1505	200,000 lb	0.94	0.94
Tensile Properties (each direction) Strength at Yield, lb/in-width Strength at Break, lb/in-width Elongation at Yield, % Elongation at Break, %	ASTM D 6693, Type IV Dumbell, 2 ipm G.L. 1.3 in (33 mm) G.L. 2.0 in (51 mm)	20,000 lb	 126 90 12 100	 N/A 120 N/A 250
Tear Resistance, lb	ASTM D 1004	45,000 lb	42	33
Puncture Resistance, lb (N)	ASTM D 4833	45,000 lb	90	66
Carbon Black Content, % (Range)	ASTM D 1603*/4218	20,000 lb	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596	45,000 lb	Note(1)	Note(1)
Asperity Height, mil	ASTM D 7466	second roll	18	18
Notched Constant Tensile Load(2), hr	ASTM D 5397, Appendix	200,000 lb	300	N/A
Oxidative Induction Time, min	ASTM D 3895, 200° C; O_2, 1 atm	200,000 lb	≥100	≥100

NOTES:
(1)Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view from Category 3. *Modified

PART 3 -- EXECUTION

3.1 STORAGE

A. After delivery, all roll goods shall be stored so as to be protected from puncture, dirt, grease, moisture and excessive heat which may result in damage or degradation of the material. Damaged material shall be stored separately for repair or replacement. The rolls shall be stored on a prepared smooth surface and should not be stacked more than two rolls high.

3.2 INSTALLATION

A. **General:** The geomembrane shall be installed in accordance with the following specifications and approved procedures submitted with the shop drawings.

B. Subgrade Preparation and Inspection:

1. Surfaces to be lined shall be smooth and free of all rocks, stones, sticks, roots, sharp objects, or debris of any kind. The surface shall provide a firm, unyielding foundation for the membrane with no sudden, sharp, or abrupt changes or break in grade.

2. The CONTRACTOR shall, on a daily basis, approve the surface on which the geomembrane shall be installed. The surface shall be smooth, clean and free of foreign material, sharp objects, frost, standing water or excessive moisture. Installation shall proceed only if the surface conditions are found satisfactory.

C. Equipment:

1. Welding equipment and accessories shall meet the following requirements:

a. Gauges showing temperatures in apparatus such as extrusion welder or fusion welder shall be present.

b. An adequate number of welding apparati shall be available to avoid delaying work.

c. Power source must be capable of providing constant voltage under combined line load.

D. Deployment:

1. Each panel shall be assigned a simple and logical identifying code.

2. The coding system shall be subject to approval by the ENGINEER and shall be determined at the job site.

3. The CONTRACTOR shall visually inspect the geomembrane during deployment for imperfections and mark faulty or suspect areas.

4. Deployment of geomembrane panels shall be performed in a manner that shall comply with the following guidelines:

a. Geomembranes shall be installed according to site-specific specifications and MANUFACTURER recommendations.

b. The geomembrane shall be placed in such a manner as to assure minimum handling.

c. Only those sheets of material which can be anchored and sealed together that same day shall be unpackaged and placed in position.

d. Deployment of the geomembrane shall proceed with ambient temperatures greater than 32° F. Placement can proceed below 32° F only after it has been verified by the ENGINEER that the material can be seamed in accordance with GRI GM9 (Cold weather seaming of geomembranes).. Placement shall not be done during any precipitation, in the presence of excessive moisture (fog, rain, dew) that deposits a residue on the liner that is detectable for sight or touch and could adversely impact the performance of the seam welding process.

e. Placement shall not be done in the presence of excessive winds which could adversely impact the ability to complete the seam welding process. In areas where wind is prevalent, installation should be started at the upwind side of the project and proceed downwind. The leading edge of the geomembrane shall be secured at all times with sandbags or other means sufficient to hold it down during high winds.

f. Geomembrane shall be unrolled using methods that shall not damage geomembrane and shall protect underlying surface from damage (spreader bar, protected equipment bucket).

g. Ballast (commonly sandbags) which shall not damage geomembrane shall be placed on geomembrane to prevent wind uplift.

h. Personnel walking on geomembrane shall not engage in activities or wear shoes that could damage it. Smoking shall not be permitted on the geomembrane.

i. No vehicle traffic shall travel on the geomembrane other than an approved low ground pressure vehicle.

j. Geomembrane shall be protected in areas of heavy traffic by placing protective cover over the geomembrane. Protective cover is material as approved by the ENGINEER that is placed over the geomembrane to reduce the ground pressure of heavy traffic to less than 8 psi on the liner.

5. Sufficient material (slack) shall be provided to allow for thermal expansion and contraction of the material.

E. Lining sheets shall be closely fitted and sealed around inlets, outlets, and other projections through the lining. Lining to concrete seals shall be made with a mechanical anchor or as approved by the ENGINEER. All piping, structures, and other projections through the lining shall be sealed with approved sealing methods.

F. Field Seams:

1. Seams shall meet the following requirements:

   a. To the maximum extent possible, seams shall be oriented parallel to line of slope, i.e., down and not across slope.

   b. The number of field seams in corners, odd-shaped geometric locations and outside corners shall be minimized.

   c. Slope seams (panels) shall extend a minimum of five-feet beyond the grade break into the flat area.

   d. Be designated using a sequential seam numbering system compatible with panel numbering system, and that is agreeable to the ENGINEER.

   e. Seam overlaps shall be aligned to be consistent with the requirements of the welding equipment being used.

2. During welding operations provide at least one Master Seamer who shall provide direct supervision over other welders as necessary.

3. Extrusion Welding

   a. Hot-air tack adjacent pieces together using procedures that do not damage the geomembrane.

   b. Clean geomembrane surfaces by disc grinder or equivalent.

   c. Purge welding apparatus of heat-degraded extrudate before welding.

4. Hot Wedge Welding

   a. Welding apparatus shall be a self-propelled device equipped with an electronic controller which displays applicable temperatures.

   b. Clean seam area of dust, mud, moisture and debris immediately ahead of hot wedge welder.

   c. Protect against moisture build-up between sheets.

5. Trial Welds

   a. Perform trial welds on geomembrane samples to verify welding equipment is operating properly.

   b. Make trial welds under the same surface and environmental conditions as the production welds, i.e., in contact with subgrade and similar ambient temperature.

   c. Minimum of two trial welds per day, per welding apparatus, one made prior to the start of work and one completed at mid shift.

   d. Cut four, one-inch wide by six-inch long test strips from the trial weld.

   e. Quantitatively test specimens for peel adhesion, and then for shear strength.

f. Trial weld specimens shall pass when the results shown in the Table 4 are achieved in both peel and shear test:

TABLE 4 – SEAM PROPERTIES

Property	Test Method	Minimum Values	
		60-mil HDPE	60-mil LLDPE
Peel Strength (fusion) ppi (1), (2)	ASTM D6392	91	75
Peel Strength (extrusion) ppi (1), (2)	ASTM D6392	78	66
Shear Strength (fusion and ext.) ppi	ASTM D6392	120	90

Notes:
1) The break, when peel testing, occurs in the geomembrane material itself, not through peel separation (FTB).
2) The break is ductile.

g. Repeat the trial weld, in its entirety, when any of the trial weld samples fail in either peel or shear.

h. No welding equipment or welder shall be allowed to perform production welds until equipment and welders have successfully completed trial weld.

i. Seaming shall not proceed when ambient air temperature or adverse weather conditions jeopardize the integrity of the geomembrane installation. CONTRACTOR shall demonstrate that acceptable seaming can be performed by completing acceptable trial welds.

j. Defects and Repairs

1) Examine all seams and non-seam areas of the geomembrane for defects, holes, blisters, undispersed raw materials, and any sign of contamination by foreign matter.

2) Repair and non-destructively test each suspect location in both seam and non-seam areas. Do not cover geomembrane at locations that have been repaired until test results with passing values are available.

G. **Anchor Trench:** The geomembrane shall be placed and secured in an earth anchor trench as indicated in the Contract Drawings. The installer shall coordinate with the earthwork contractor regarding excavation and backfilling of the anchor trench. Care shall be taken when backfilling the trenches to prevent any damage to the geomembrane. If damage occurs, it shall be repaired prior to backfilling.

3.3 SEAM TESTING

A. Field Destructive Testing

1. Destructive seam tests shall be performed to evaluate bonded seam strength. The frequency of sample removal shall be one sample per 500 linear feet of seam. Location of the destructive samples shall be selected by the ENGINEER. Field testing shall take place as soon as possible after completion of the seam.

   a. At the sole discretion of the ENGINEER, destructive seam tests may be reduced in frequency by following the procedures of Geosynthetic Research Institute (GRI) Standard Guide GM 14.

2. Sample labeling shall be the responsibility of the ENGINEER and shall include test number, seam number, seaming machine number, job number, date welded, and welding tech number.

3. The samples shall be approximately 12 inches x 25 inches. The samples shall then be cut into two samples approximately 12 inches x 12 inches: one for field testing and one for archiving or independent testing.

4. The sample for field testing shall have ten coupons cut and be tested with a tensiometer adjusted to a pull rate as shown below. The strength of four out of five specimens shall meet or exceed the values below, and the fifth value must meet or exceed 80% of the value below.

   a. Seam must exhibit film tear bond (FTB). Welds shall have less than 25% incursion into the weld.

   b. Peel and shear values shall meet or exceed the values in Table 4 (at 2 inches/minute)

5. All destructive weld test data shall be logged by the ENGINEER.

6. If a test fails, additional samples shall be cut, approximately ten feet on each side of the failed test, and retested. This procedure shall be repeated until a sample passes. Then the area of the failed seam between the two tests that pass shall be capped or reconstructed.

B. Non-Destructive Testing

1. The CONTRACTOR shall non-destructively test all seams their full length for continuity using an air pressure or vacuum test.

2. Air Pressure Testing

   a. Air pressure testing shall be performed on all seams welded with a double seam fusion welder.

   b. The equipment used for air pressure testing shall consist of an air tank or pump capable of producing a minimum of 35 psi and a sharp needle with a pressure gauge attached to insert into the air chamber.

   c. Both ends of the seam to be tested shall be heated and squeezed together.

d. The needle with gauge shall be inserted into the air channel and the channel shall be pressurized to 30 psig.

e. If the pressure in the air channel drops by more than 4 psig over a period of five minutes, then the seam has failed.

f. If the seam fails the air pressure test, the leak shall be located and the area cut away. Air pressure testing shall be performed on the remaining portions of the seam until all portions of the seam pass the test.

g. The area cut away shall be repaired with a patch. The patch shall be tested according to the procedures outlined below for vacuum testing.

3. Vacuum Testing

a. Vacuum testing shall be performed on all seams welded with an extrusion welder.

b. The equipment used for vacuum testing shall consist of a vacuum pumping device, a vacuum box, and a foaming agent in solution.

c. The section of seam to be tested shall be wetted with a foaming agent and the vacuum box shall be placed over the wetted area. Air shall be evacuated from the vacuum box until a seal between the box and the geomembrane has been formed.

d. The minimum vacuum shall be equivalent to 5 psig (10 inches of mercury).

e. If fusion welded seams are being tested, the overlap flap must be cut off prior to testing.

f. The seam shall be observed through the viewing window for bubbles emitting from the seam.

g. If no bubbles are observed, the box shall be moved on to the next area for testing. If bubbles are observed, the area of the leak shall be marked for repair.

h. After completion of repairs, the repair seam shall be retested according to the requirements of paragraph 3.3B.

3.4 INSPECTION AND REPAIR

A. **Field Inspection:** All seals to penetrations as well as all seams and non-seam areas of the geomembrane shall be inspected for defects, holes, blisters, undispersed raw materials, and any sign of contamination by foreign matter. The surface of the geomembrane shall be clean at the time of inspection. Each suspect location shall be non-destructively tested as appropriate and repaired accordingly.

B. Repair Procedures:

1. Remove damaged geomembrane and replace with acceptable geomembrane materials if damage cannot be satisfactorily repaired.

2. Repair any portion of unsatisfactory geomembrane or seam area failing a destructive or non-destructive test.

3. CONTRACTOR shall be responsible for repair of defective areas.

4. Agreement upon the appropriate repair method shall be decided between ENGINEER and CONTRACTOR by using one of the following repair methods:

   a. Patching- Used to repair large holes, tears, undispersed raw materials and contamination by foreign matter.

   b. Abrading and Re-welding- Used to repair short section of a seam.

   c. Spot Welding- Used to repair pinholes or other minor, localized flaws or where geomembrane thickness has been reduced.

   d. Capping- Used to repair long lengths of failed seams.

   e. Flap Welding- Used to extrusion weld the flap (excess outer portion) of a fusion weld in lieu of a full cap.

   f. Remove the unacceptable seam and replace with new material.

5. The following procedures shall be observed when a repair method is used:

   a. All geomembrane surfaces shall be clean and dry at the time of repair.

   b. Surfaces of the polyethylene which are to be repaired by extrusion welds shall be lightly abraded to assure cleanliness.

   c. Extend patches or caps at least 6 inches for extrusion welds and 4 inches for wedge welds beyond the edge of the defect, and around all corners of patch material.

6. Repair Verification

   a. Number and log each patch repair (performed by ENGINEER).

   b. Non-destructively test each repair using methods specified in this Specification.

7. The CONTRACTOR shall also keep detailed record drawings showing the location, size, type, and frequency of all repairs made during the installation of the geomembrane. These record drawings shall be updated by the CONTRACTOR on a daily basis and submitted to the COMPANY upon completion of the project. Inspection of these record drawings shall be made available to the ENGINEER or the COMPANY for verification and review at any time during the construction period.

3.5 ACCEPTANCE

A. The CONTRACTOR shall retain all ownership and responsibility for the geomembrane system until acceptance by the ENGINEER. Final acceptance shall occur when the following conditions are met:

   1. Installation is finished.

2. Verification of the adequacy of all field seams and repairs is complete.

3. Submittals required in Paragraph 1.2 D have been accepted by the ENGINEER.

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SECTION 02930 - SEEDING

PART 1 -- GENERAL

1.1 SUMMARY

A. The Contractor shall apply reclamation seed mix to the completed cover, complete and in place, in accordance with the Contract Documents.

1.2 REFERENCE SPECIFICATIONS, CODES, AND STANDARDS

A. Federal Specifications:

FS O-F-241D	Fertilizer, Mixed, Commercial.

B. Commercial Standards:

ANSI/ASTM D 422	Method for Particle-size Analysis of Soils.

1.3 CONTRACTOR SUBMITTALS

A. Furnish submittals for approval.

B. Materials List: A list of all materials to be used in the seeding operations together with the source of those materials. The list shall include mulches, soil amendments, seed mixtures, and erosion control blanketing. Manufacturer's literature showing physical characteristics, applications, and installation instrumentation shall be included.

C. Schedules: The following work plans, before work is started.

1. Delivery schedule at least 10 days prior to the intended date of the first delivery.

2. Seeding Operation: A list of seeding and mulching equipment to be used.

D. Reports

1. Certified reports of inspections and laboratory tests, prepared by an independent testing agency, including analysis and interpretation of test results. Each report shall be properly identified. Test methods used and compliance with recognized test standards shall be described.

2. Reports for the following materials shall be included.

a. Fertilizer: For chemical analysis and composition percent.

b. Seed: For mixture, percent pure live seed, minimum percent germination and hard seed, maximum percent weed content, date tested and state certification.

E. Certificates: Certificates of compliance that materials meet the indicated requirements prior to the delivery of materials.

F. Records:

1. Plant Establishment Period

2. Maintenance Report

3. Maintenance Instructions

1.4 CLEANUP

A. Upon completion of all seeding operations, the portion of the Site used for a work or storage area by the Contractor shall be cleaned of all debris, superfluous materials, equipment, and garbage.

1.5 MAINTENANCE OF LANDSCAPING PLANTING PRIOR TO ACCEPTANCE OF PROJECT

A. General: The Contractor shall be responsible for protecting seeded areas until final acceptance of the Work.

B. Upon completion of seeding, the entire planted area shall be soaked to saturation by a fine spray. Care shall be taken to avoid excessive washing, or puddling on the surface, and any such damage caused thereby shall be repaired by the Contractor.

C. Protection: The Contractor shall provide adequate protection to all newly seeded areas including the installation of approved temporary fences to prevent trespassing and damage, as well as erosion control, until the end of the one-year warranty period.

1.6 FINAL INSPECTION AND GUARANTEE

A. Inspection of seeded areas will be made at final acceptance

B. Written notice requesting inspection shall be submitted to the Engineer at least 10 days prior to the anticipated inspection date.

C. Any delay in completing the Work of this Section beyond a single season will be cause for extending the correction of defects period an equal time.

D. The Contractor shall, without additional expense to the Company, replace seeding which develops defects or dies during the correction period.

PART 2 -- PRODUCTS

2.1 GENERAL

A. Cover soil shall be obtained from onsite borrow sources.

2.2 TOPSOIL

A. General fill and clay to be blended to generate the topsoil shall be obtained from the pre-established borrow source at a location directed by the Company and placed in accordance with Section 02222 – Earthwork and Grading.

2.3 FERTILIZER AND ADDITIVES

A. Fertilizer shall be furnished in bags or other standard containers with name, weight, and guaranteed analysis of contents clearly marked thereon.

B. Fertilizers shall be uniform in composition, dry, and free flowing.

C. Chemical fertilizers shall be a mixed uncommercial fertilizer with nitrogen (N), phosphorous (P), and potassium (K) at the following application rates. Nitrogen shall be applied at 70 lbs /acre in the form of ammonium nitrate (33-0-0). This is an equivalent of 23 lbs of total N/acre (70 x 33%). Phosphorous shall be applied at 150 lbs/acre in the form of triple super phosphate (0-44-0). This form of phosphorous contains 20% total P, so the application of total P will be 30 lbs/acre. Potassium shall be applied at 60 lbs/acre in the form of potassium chloride (0-0-60). This form of potassium contains 50% total K, so the application of total K will be 30 lbs/acre. Fertilizer recommendations may be modified as to the forms or blends of fertilizer used as formulations vary by region. The total nutrient application rate for each of the nutrients shall be matched within ±10% of what is recommended. Fertilizers shall be uniform in composition, dry, and free flowing.

2.4 MULCH

A. Wood Cellulose Fiber: shall not contain any growth or germination-inhibiting factors and shall be dyed an appropriate color to aid visual monitoring during application. Composition will include at least 70 percent specially prepared virgin cellulose fiber and shall contain the following properties: recycled cellulose fiber (30 percent minimum), ash content (0.8 to 1.1 percent maximum), water holding capacity (10 to 1 ratio of water to fiber), and pH range from 4.5 to 5.5.

B. Weed free straw mulch, or native hay, for a soil/seed stabilizer shall be clean hay or straw applied at a rate of 2 tons per acre. Mulch shall be crimped into soil with a mulch crimper. Spacing on the blades of the mulch crimper shall be 6-inches minimum and 9-inches maximum. Blades shall be sufficiently weighted to penetrate the ground 3 inches.

2.5 SEED MIXTURES

A. All seed shall conform to applicable County, State of Utah, and Federal regulations. Seed shall be mixed by the seed supplier. The Contractor shall furnish the seed supplier's guaranteed germination of each variety listed in the seed mixture. Grass seed shall not be delivered to the Site until samples have been approved by the Engineer. Approval of samples, however, shall not affect the right of the Engineer to reject seed upon or after delivery. Seed which has become wet, moldy, or otherwise damaged prior to use will not be accepted.

B. Seed shall be delivered in strong, clearly marked bags not exceeding 50 pounds each.

C. Seed shall be fresh, clean, and new-crop seed composed of the following varieties mixed in the proportions by weight as indicated. Seed shall be tested for compliance with the minimum percentage of purity and germination requirements. All rates specified shall be pure live seed (PLS).

D. The seed mixture shall not contain more than 5 percent weeds or other species that are not required.

E. Any deviation of the indicated seed mixture composition shall be approved by the Engineer prior to delivery.

SEED MIXTURE	
Common Names	**Drill Seeding Rate (lbs pf Pure Live Seed/Acre)**
Tall Wheatgrass	2.0
Hercules Tall Wheatgrass	2.0
AC Saltlander Green Wheatgrass	4.0
Garrison Creeping Foxtail	2.5
Intermediate Wheatgrass	2.5
FSG423ST Salt Tolerant Alfalfa	1.5
Strawberry Clover	1.5
Total	16.0

PART 3 -- EXECUTION

3.1 GENERAL

A. Delivery of seed and fertilizer may begin only after samples and tests have been approved by the Engineer. Seed and fertilizer furnished shall not be different from the approved sample.

B. Seeding shall not be performed at any time when it may be impaired by climatic conditions.

3.2 SOIL PREPARATION

A. The seeding shall not begin until the Contractor has repaired all areas of settlement, erosion, rutting, etc. and the soils have been placed, compacted, and contoured to finish grade. The Engineer shall be notified of areas that prevent the planting work from being executed.

B. After removal of waste materials in the planting areas, such as weeds, roots, rocks 6 inches and larger, construction materials, etc., the seeding subgrade shall be tilled to a depth of 6 inches and all surface irregularities removed.

C. Areas requiring grading by the Contractor including adjacent transition areas shall be uniformly level or sloping between finish elevations to within 0.10-ft above or below required finish elevations.

D. Any unusual subsoil condition that will require special treatment shall be reported to the Engineer.

E. Topsoil: Topsoil shall be placed in accordance with Section 02222 – Earthworks and Grading. Topsoil shall not be placed when the subgrade is frozen, excessively wet, extremely dry, excessively compacted or in a condition detrimental to the proposed planting or grading.

F. Fertilizer: Fertilizer shall be applied at the following rates:

1. Nitrogen shall be applied at 70 lbs /acre in the form of ammonium nitrate (33-0-0). This is an equivalent of 23 lbs of total N/acre (70 x 33%).

2. Phosphorous shall be applied at 150 lbs/acre in the form of triple super phosphate (0-44-0). This form of phosphorous contains 20% total P, so the application of total P will be 30 lbs/acre.

3. Potassium shall be applied at 60 lbs/acre in the form of potassium chloride (0-0-60). This form of potassium contains 50% total K, so the application of total K will be 30 lbs/acre.

G. Fertilizer shall be incorporated into the soil to a minimum depth of 6 inches and may be incorporated as part of the tillage operation.

H. Tillage

1. Preparation. Seed areas shall be filled as needed or have surplus soil removed to attain the finished grade. Drainage patterns shall be maintained as indicated on drawings. Seed areas compacted by construction operations shall be completely pulverized by tillage.

2. Protection. Finished graded areas shall be protected from damage by vehicular or pedestrian traffic and erosion.

3. Finish Grading. Finished grade shall be 1-inch below the adjoining grade of any surfaced area. New surfaces shall be blended to existing. Make minor adjustments of finish grades as directed by the Engineer.

I. No seeding shall be done when wind velocity exceeds 4 mph, within 4 hours after rain, or if the surface has been compacted without first loosening the ground.

3.3 HYDROSEEDING

A. **Equipment:** Mixing shall be performed in a tank. The tank shall have a built-in continuous agitation and circulation system, of sufficient operating capacity to produce a homogenous slurry of mulch, stabilizer, seed, fertilizer and water in the designated unit proportions for a minimum coverage of one-half acre. The tank shall have a discharge system which will permit attachment of at least 500-feet of hose extensions, a change of elevation of 150-feet in height from tank to discharge nozzle, and still retain enough pressure to apply the slurry to the areas at a continuous and uniform rate.

B. **Proportions:** Proportions of mulch, seed, stabilizer and water per acre shall be as indicated in the approved Revegetation Plan, or as otherwise approved by the ENGINEER.

C. Application

1. With agitation system operating at part speed, water shall be added to the tank and good recirculation shall be established. Materials shall be added in such a manner that they are uniformly blended into the mixture.

2. Slurry distribution shall begin immediately. Application of slurry shall be done only when rain is not anticipated for at least three days after slurry application.

3. The entire tank of each batch of slurry shall be emptied and the slurry evenly applied to areas to be hydroseeded within a 2 hour period following the mixing of each slurry batch. Slurry batches not applied during this time will be rejected.

3.4 DRILL SEEDING

A. **Equipment:** Seeding drill shall be a mechanical grass drill with depth bands and have multiple seed boxes to appropriate to the size and weight of the specified seeds.

B. All seed shall be drilled to one-quarter (1/4) inch to one half (1/2) inch into the soil at the specified seed rate.

C. CONTRACTOR shall drill on-half (1/2) of the required seed in one direction, and then drill the remaining half of the required seed in a direction 90° to the first half.

3.5 SEEDING COMPLETION

A. Mulching: Immediately after seeding, the entire area shall be mulched with one of the two following methods:

1. Weed free straw or native hay at a rate of 2 tons per acre. Weed free straw mulch or native hay for a soil/seed stabilizer shall be clean hay or straw. Mulch shall be crimped into soil with a mulch crimper. Spacing on the blades of the mulch crimper shall be 6-inches minimum and 9-inches maximum. Blades shall be sufficiently weighted to penetrate the ground 3-inches.

2. Hydromulching with wood fiber mulch can be used as an alternative to straw or hay and applied at a rate of 1.5 tons/acre along with a tackifier to bind the mulch to the soil.

3.4 INSPECTION

A. At the completion of the work, the Contractor shall request a preliminary inspection by the Engineer to determine the condition of seeding.

B. A final inspection shall be requested 48 hours following seed germination. The Contractor and Engineer will be present for the inspection. Seeded areas considered for final inspection shall show uniform smooth ground surface without eroded ruts or gullies and evidence of uniform seed germination.

3.5 ACCEPTANCE

A. If the installation is found satisfactory, the Company will approve the work in writing.

B. If the installation is found unsatisfactory, the Engineer will submit a punch list of conditions to correct at the Contractor's expense. The Contractor shall be responsible for requesting additional inspections after the conditions of the punch list have been corrected.

C. The final acceptance criteria for seeding will be an average of one seedling (from seeded species) per square foot after the first growing season. Therefore, for seeding performed in late fall, the evaluation of final acceptance will be determined in the fall of next year.

D. Any areas not achieving the acceptance criteria presented above will be re-seeded at the expense of the Contractor.

3.6 REPAIRS

A. Seed shall be re-applied in any area, including washout gullies and/or slopes, where growth has not initiated during the first rainy season, November through April, following initial application. Washout gullies will require the placement of additional topsoil to fill washouts in accordance with Section 02222 – Earthwork and Grading, prior to re-seeding.

- END OF SECTION -

Appendix C

Borrow Area 3 Test Pit Logs

TRENCH TEST PIT LOG FORM

Page 1 of 1

Project IPSC CCR Closures Project Number 233001396

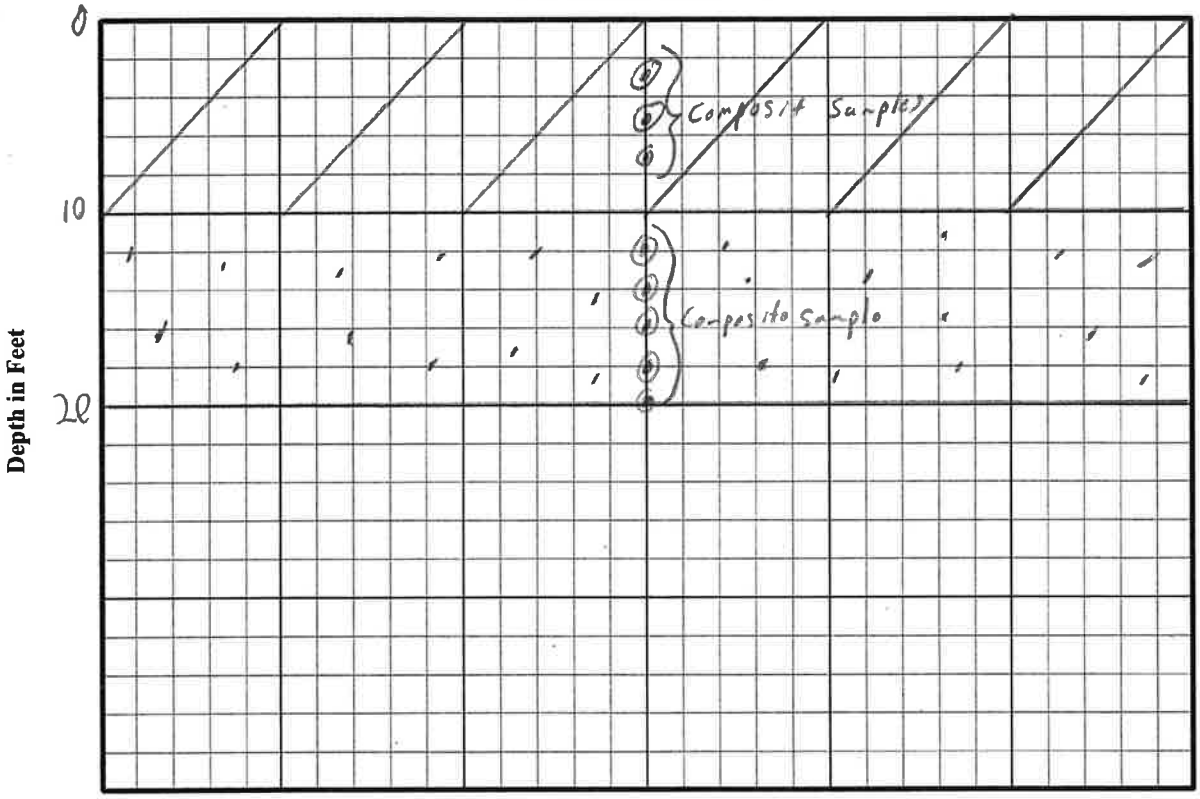
Sample Location Borrow Area 3 Trench Number B3TP-1 Date 10/29/20

Coordinates: Inside Stake Outside Stake

Native/Fill Stake

Logged By Chad Tomlinson

TRENCH PROFILE



Subsurface description and filed USCS Classifications
(USCS name, color, size and angularity or plasticity, density, moisture content, additional facts and debris encountered)

0-10' - Light brown in color, clayey sand (SC), low plasticity

10-20' - Light brown in color, transitioning to clay with sand (CL), moderate plasticity

Begin Trench 10:20 Finish Trench 10:35 Trenching Contractor IPSC

Total Depth 20' Total Length 10'

TRENCH TEST PIT LOG FORM

Page 1 of 1

Project IPSC CCR Closure | Project Number 233001396

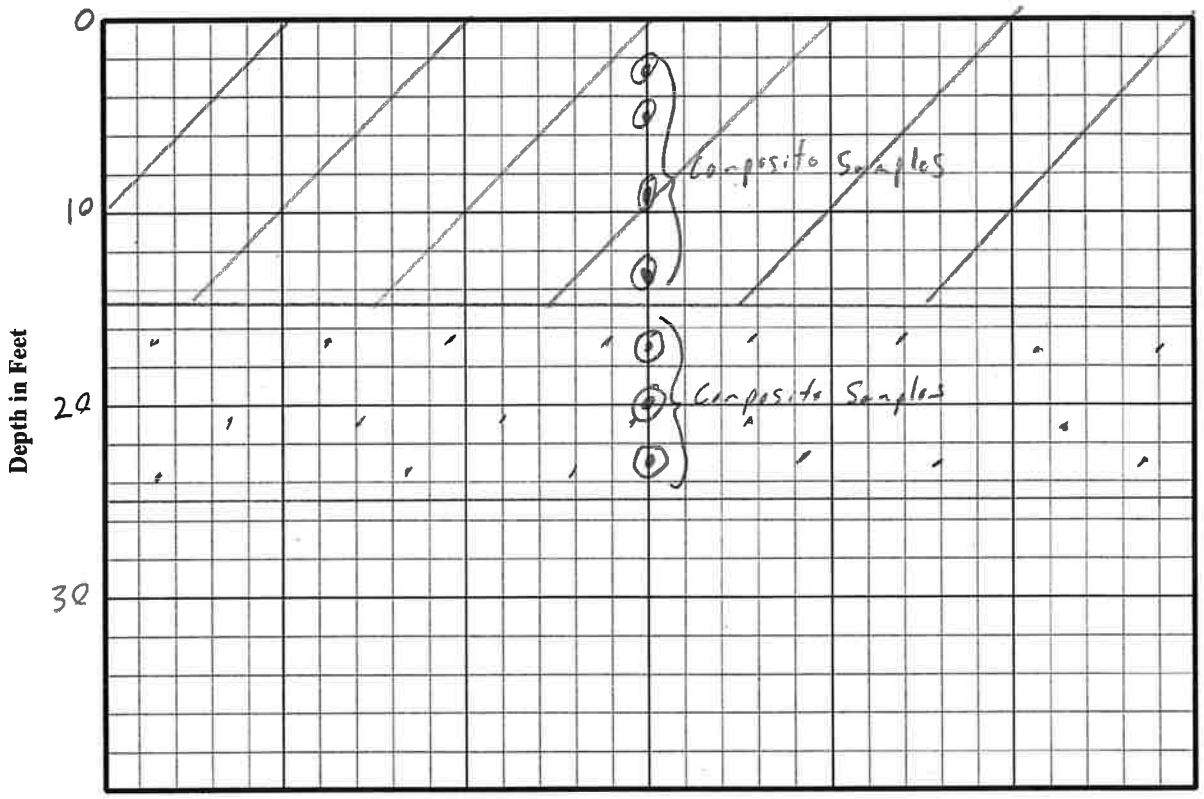
Sample Location Borrow Area 3 | Trench Number B3TP-2 | Date 10/29/19

Coordinates: Inside Stake | Outside Stake

Native/Fill Stake

Logged By Chad Tomlinson

TRENCH PROFILE



Subsurface description and filed USCS Classifications
(USCS name, color, size and angularity or plasticity, density, moisture content, additional facts and debris encountered)

0-15' - light brown, sandy Clay, moderate plasticity

15-25' - Same as above.

Begin Trench 10:40 | Finish Trench 10:50 | Trenching Contractor IPSC

Total Depth 25' | Total Length 10'

TRENCH TEST PIT LOG FORM

Project IPSC CCR Closures Project Number 233001396

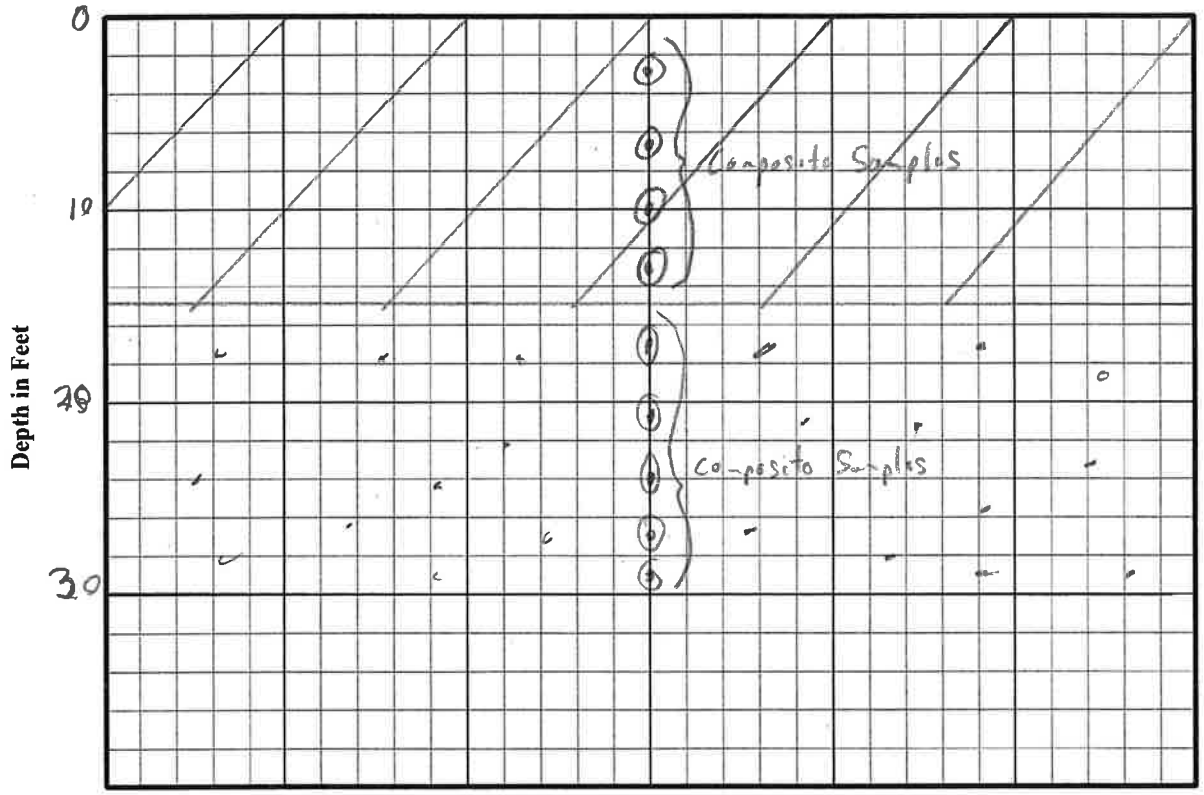
Sample Location Borrow Area 3 Trench Number B3TP-3 Date 10/29/20

Coordinates: Inside Stake Outside Stake

Native/Fill Stake

Logged By Chad Tomlinson

TRENCH PROFILE



Subsurface description and filed USCS Classifications
(USCS name, color, size and angularity or plasticity, density, moisture content, additional facts and debris encountered)

0-15' - Light brown, silty sand, low to no plasticity

15-30' - Light brown, transition from silty sand to clayey sand, moderate plasticity

Begin Trench 10:55 Finish Trench 11:15 Trenching Contractor IPSC

Total Depth 30' Total Length 10'

Appendix D

Laboratory Test Results

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-1
Depth: 0-10'
Description: Lean CLAY, brown

Preparation method: Wet
Liquid limit test method: Multipoint
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: 3/4"
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

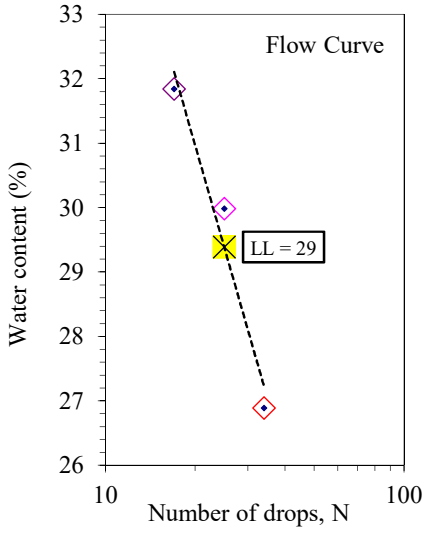
Plastic Limit

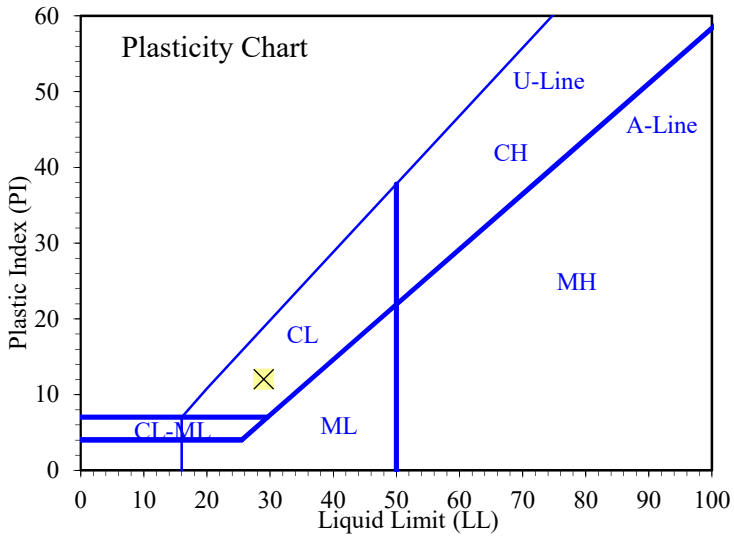
Determination No	1	2				
Wet Soil + Tare (g)	14.37	14.62				
Dry Soil + Tare (g)	13.28	13.51				
Water Loss (g)	1.09	1.11				
Tare (g)	7.08	7.11				
Dry Soil (g)	6.20	6.40				
Water Content, w (%)	17.58	17.34				

Liquid Limit

Determination No	1	2	3			
Number of Drops, N	34	25	17			
Wet Soil + Tare (g)	14.56	15.45	16.23			
Dry Soil + Tare (g)	13.03	13.66	14.10			
Water Loss (g)	1.53	1.79	2.13			
Tare (g)	7.34	7.69	7.41			
Dry Soil (g)	5.69	5.97	6.69			
Water Content, w (%)	26.89	29.98	31.84			
One-Point LL (%)		30				

Liquid Limit, LL (%)	**29**
Plastic Limit, PL (%)	**17**
Plasticity Index, PI (%)	**12**





Entered by:_____
Reviewed:_____

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-1
Depth: 10-20'
Description: Lean CLAY, brown

Preparation method: Wet
Liquid limit test method: Multipoint
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: 3/8"
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

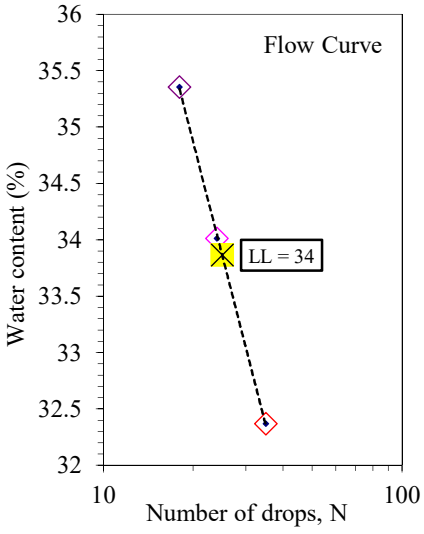
Plastic Limit

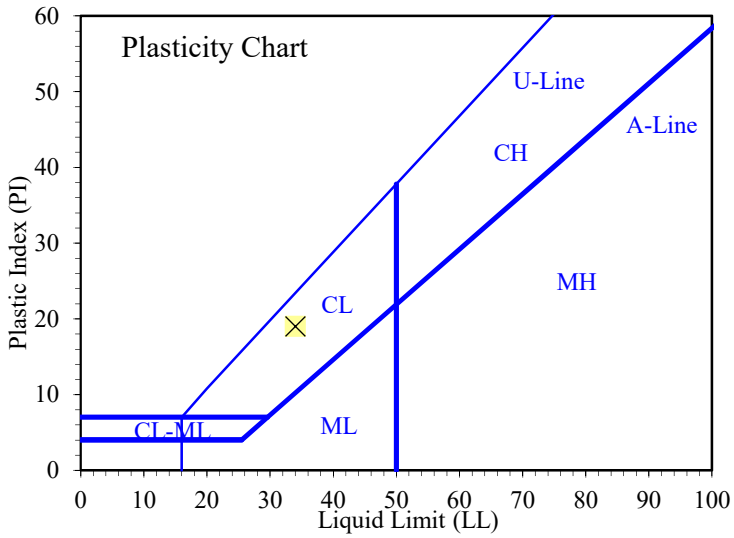
Determination No	1	2				
Wet Soil + Tare (g)	14.56	14.71				
Dry Soil + Tare (g)	13.56	13.73				
Water Loss (g)	1.00	0.98				
Tare (g)	7.03	7.11				
Dry Soil (g)	6.53	6.62				
Water Content, w (%)	15.31	14.80				

Liquid Limit

Determination No	1	2	3			
Number of Drops, N	35	24	18			
Wet Soil + Tare (g)	15.70	16.50	15.24			
Dry Soil + Tare (g)	13.69	14.33	13.20			
Water Loss (g)	2.01	2.17	2.04			
Tare (g)	7.48	7.95	7.43			
Dry Soil (g)	6.21	6.38	5.77			
Water Content, w (%)	32.37	34.01	35.36			
One-Point LL (%)		34				

Liquid Limit, LL (%)	**34**
Plastic Limit, PL (%)	**15**
Plasticity Index, PI (%)	**19**





Entered by:_____
Reviewed:_____

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-2
Depth: 0-15'
Description: Lean CLAY, brown

Preparation method: Wet
Liquid limit test method: Multipoint
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: 3/8"
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

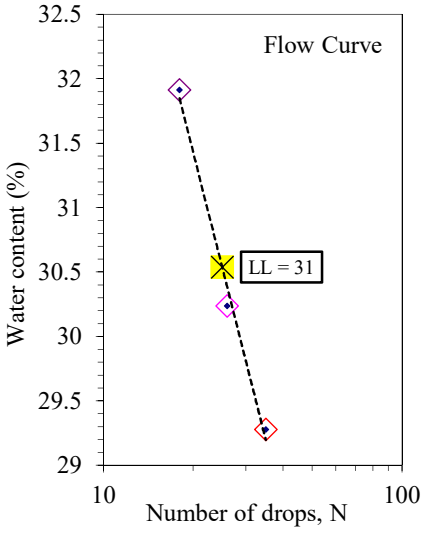
Plastic Limit

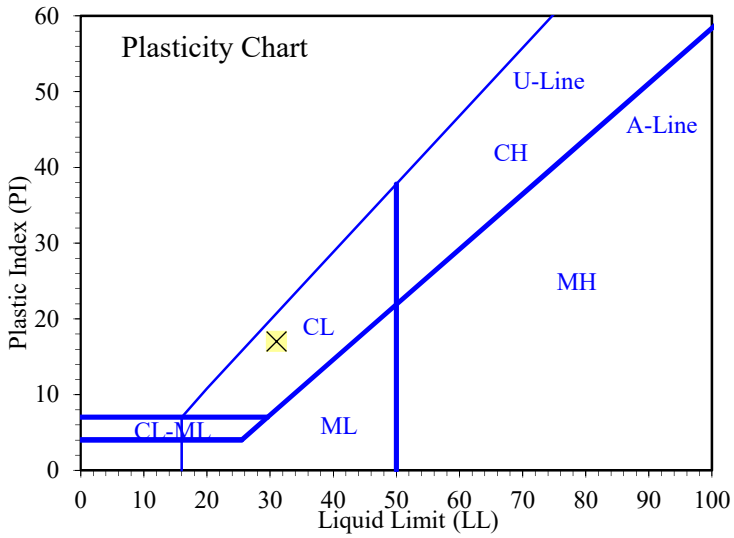
Determination No	1	2				
Wet Soil + Tare (g)	13.77	13.08				
Dry Soil + Tare (g)	12.94	12.34				
Water Loss (g)	0.83	0.74				
Tare (g)	7.05	7.03				
Dry Soil (g)	5.89	5.31				
Water Content, w (%)	14.09	13.94				

Liquid Limit

Determination No	1	2	3			
Number of Drops, N	35	26	18			
Wet Soil + Tare (g)	15.47	14.82	16.03			
Dry Soil + Tare (g)	13.57	13.03	13.86			
Water Loss (g)	1.90	1.79	2.17			
Tare (g)	7.08	7.11	7.06			
Dry Soil (g)	6.49	5.92	6.80			
Water Content, w (%)	29.28	30.24	31.91			
One-Point LL (%)		30				

Liquid Limit, LL (%)	**31**
Plastic Limit, PL (%)	**14**
Plasticity Index, PI (%)	**17**





Entered by:_____
Reviewed:_____

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-2
Depth: 15-25'
Description: Lean CLAY, brown

Preparation method: Wet
Liquid limit test method: Multipoint
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: 3/8"
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

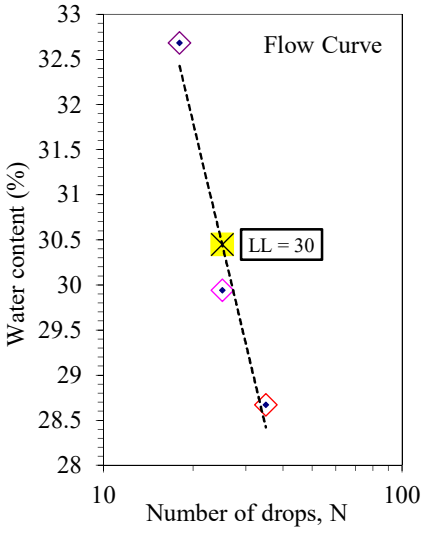
Plastic Limit

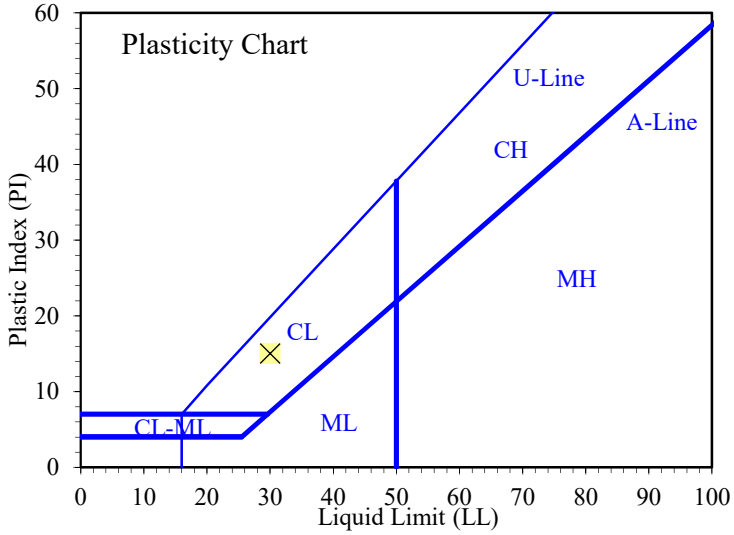
Determination No	1	2				
Wet Soil + Tare (g)	13.81	14.54				
Dry Soil + Tare (g)	12.93	13.56				
Water Loss (g)	0.88	0.98				
Tare (g)	7.03	7.13				
Dry Soil (g)	5.90	6.43				
Water Content, w (%)	14.92	15.24				

Liquid Limit

Determination No	1	2	3			
Number of Drops, N	35	25	18			
Wet Soil + Tare (g)	14.98	16.02	14.94			
Dry Soil + Tare (g)	13.30	14.02	13.10			
Water Loss (g)	1.68	2.00	1.84			
Tare (g)	7.44	7.34	7.47			
Dry Soil (g)	5.86	6.68	5.63			
Water Content, w (%)	28.67	29.94	32.68			
One-Point LL (%)		30				

Liquid Limit, LL (%)	**30**
Plastic Limit, PL (%)	**15**
Plasticity Index, PI (%)	**15**





Entered by:_____
Reviewed:_____

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-3
Depth: 0-15'
Description: SILT, brown

Preparation method: Wet
Liquid Limit: Could not be determined (N.P.)
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: 3/8"
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

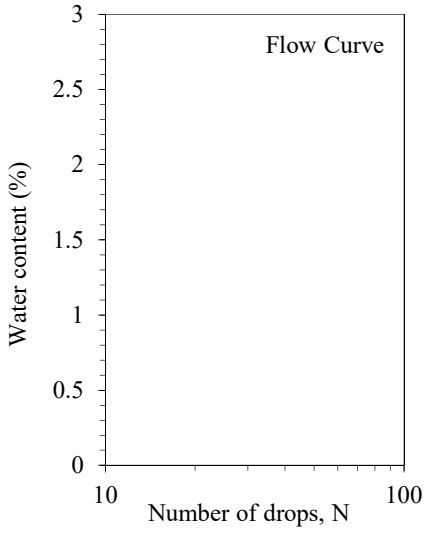
Plastic Limit

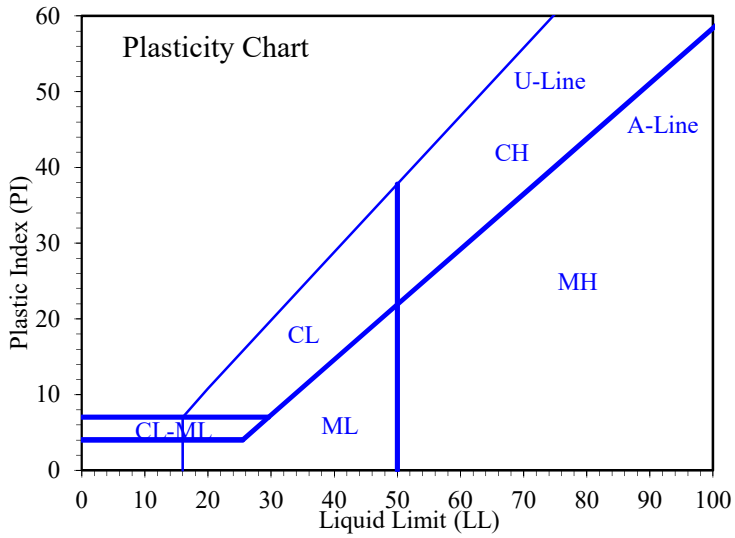
Determination No						
Wet Soil + Tare (g)						
Dry Soil + Tare (g)		Difficult to thread.				
Water Loss (g)						
Tare (g)						
Dry Soil (g)						
Water Content, w (%)						

Liquid Limit: Could not be determined (N.P.)

Determination No						
Number of Drops, N						
Wet Soil + Tare (g)	Unable to obtain an adequate blow count.					
Dry Soil + Tare (g)						
Water Loss (g)						
Tare (g)						
Dry Soil (g)						
Water Content, w (%)						
One-Point LL (%)						

Liquid Limit, LL (%)	**Nonplastic (N.P.)**
Plastic Limit, PL (%)	
Plasticity Index, PI (%)	





Entered by:_____
Reviewed:_____

Liquid Limit, Plastic Limit, and Plasticity Index of Soils

(ASTM D4318)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: BRR
Grooving tool type: Plastic
Liquid limit device: Mechanical
Rolling method: Hand

Boring No.:
Sample: B3TP-3
Depth: 15-30'
Description: Lean CLAY, brown

Preparation method: Wet
Liquid limit test method: Multipoint
Screened over No.40: Yes
Larger particles removed: Wet sieved
Approximate maximum grain size: No.4
Estimated percent retained on No.40: See Particle Size Distribution
As-received water content (%): Not requested

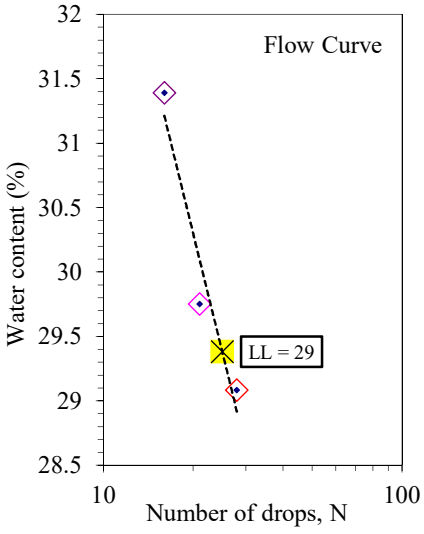
Plastic Limit

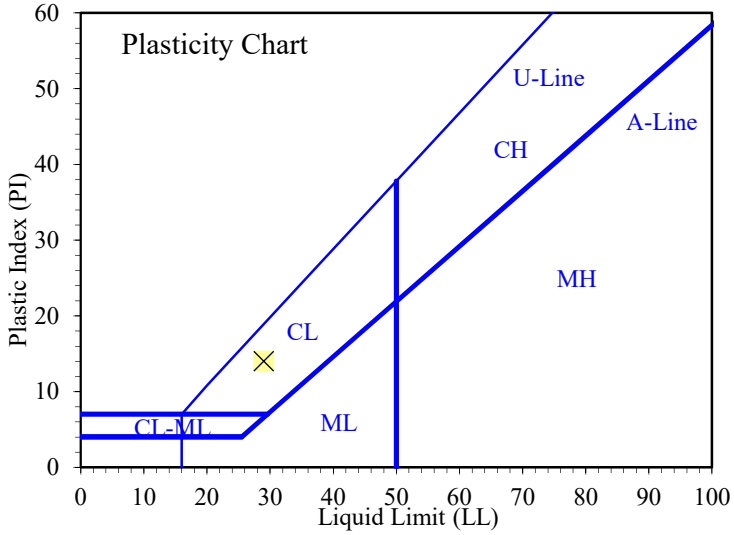
Determination No	1	2				
Wet Soil + Tare (g)	13.22	13.61				
Dry Soil + Tare (g)	12.41	12.75				
Water Loss (g)	0.81	0.86				
Tare (g)	7.12	7.07				
Dry Soil (g)	5.29	5.68				
Water Content, w (%)	15.31	15.14				

Liquid Limit

Determination No	1	2	3			
Number of Drops, N	28	21	16			
Wet Soil + Tare (g)	13.54	13.77	17.19			
Dry Soil + Tare (g)	12.08	12.22	14.93			
Water Loss (g)	1.46	1.55	2.26			
Tare (g)	7.06	7.01	7.73			
Dry Soil (g)	5.02	5.21	7.20			
Water Content, w (%)	29.08	29.75	31.39			
One-Point LL (%)	29	29				

Liquid Limit, LL (%)	**29**
Plastic Limit, PL (%)	**15**
Plasticity Index, PI (%)	**14**





Entered by:_____
Reviewed:_____

Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: JAB/EH/BRR

Boring No.:
Sample: B3TP-1
Depth: 0-10'
Description: Clayey SAND, brown

ASTM Standard(s)	ASTM D6913 and ASTM D7928	
Split:	Yes	
First Split sieve:	3/8"	
Second split:	No	
	Moist	Dry
Total sample wt. (g):	9915.5	9258.2
+3/8" Coarse fraction (g):	139.95	137.45
-3/8" Split fraction (g):	368.30	343.63
Hydrometer fraction (g):	65.30	58.84
First Split fraction:	0.985	

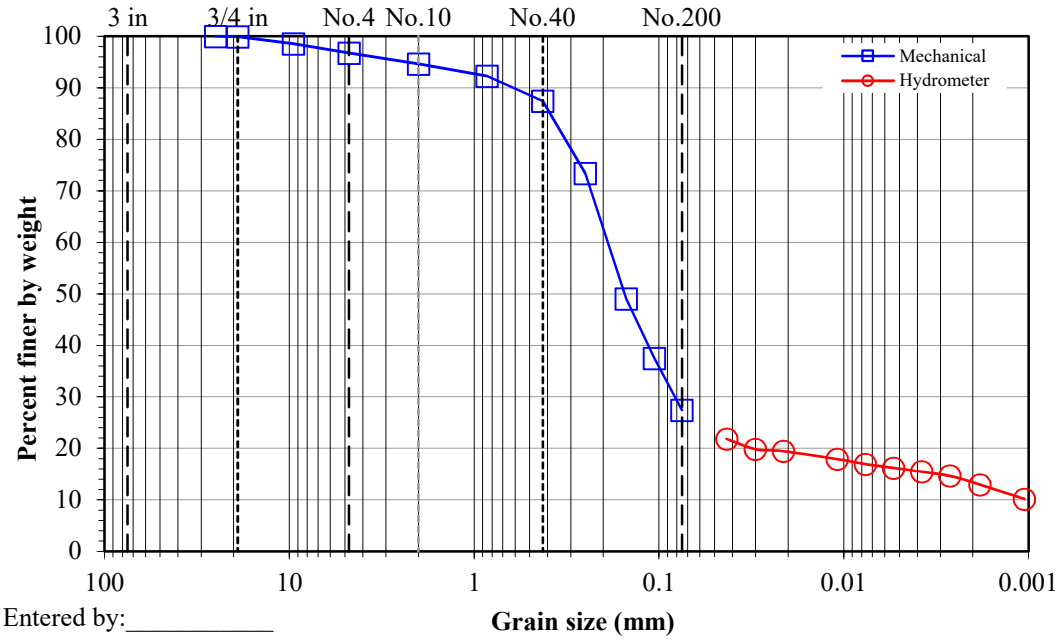
Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer
6"		150	-
4"		100	-
3"		75	-
1.5"		37.5	-
1"		25	100.0
3/4"	5.88	19	99.9
3/8"	137.45	9.5	98.5
No.4	6.25	4.75	96.7
No.10	13.50	2	94.6
No.20	21.74	0.85	92.3
No.40	38.63	0.425	87.4
No.60	87.59	0.25	73.4
No.100	172.67	0.15	49.0
No.140	212.92	0.106	37.5
No.200	247.98	0.075	27.4

Water content data	C.F.1(+3/8")	S.F.1(-3/8")	Hyd.(-No.10)
Moist soil + tare (g):	264.47	497.11	30.00
Dry soil + tare (g):	261.94	472.44	27.73
Tare (g):	123.06	128.81	7.06
Water content (%):	1.82	7.18	10.98

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.7	Assumed	
Bulb No.	6	Hyd. fraction:	94.65
Cylinder ID:	T5	Dispersion device:	Air-jet

	Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
	1	22.1	18.5	0.0430	21.83
	2	22.1	17.25	0.0301	19.84
	4	22.1	17	0.0212	19.44
	15	22.2	16	0.0109	17.91
	30	21.9	15.5	0.0077	16.94
	60	22	15	0.0054	16.20
	120	22.1	14.5	0.0038	15.47
	240	22.1	14	0.0027	14.67
	500	21.9	13	0.0018	12.97
<=1st Split	1465	21.9	11.25	0.0011	10.18
<=Split hyd.					

Gravel (%): 3.3
Sand (%): 69.3
Fines (%): 27.4



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Reviewed:_____

Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: JAB/EH/BRR

Boring No.:
Sample: B3TP-1
Depth: 10-20'
Description: Lean CLAY with sand, brown

ASTM Standard(s)	ASTM D6913 and ASTM D7928	
Split:	Yes	
First Split sieve:	3/8"	
Second split:	No	
	Moist	Dry
Total sample wt. (g):	9285.1	7989.9
+3/8" Coarse fraction (g):	18.33	17.92
-3/8" Split fraction (g):	308.83	265.68
Hydrometer fraction (g):	65.18	57.28
First Split fraction:	0.998	

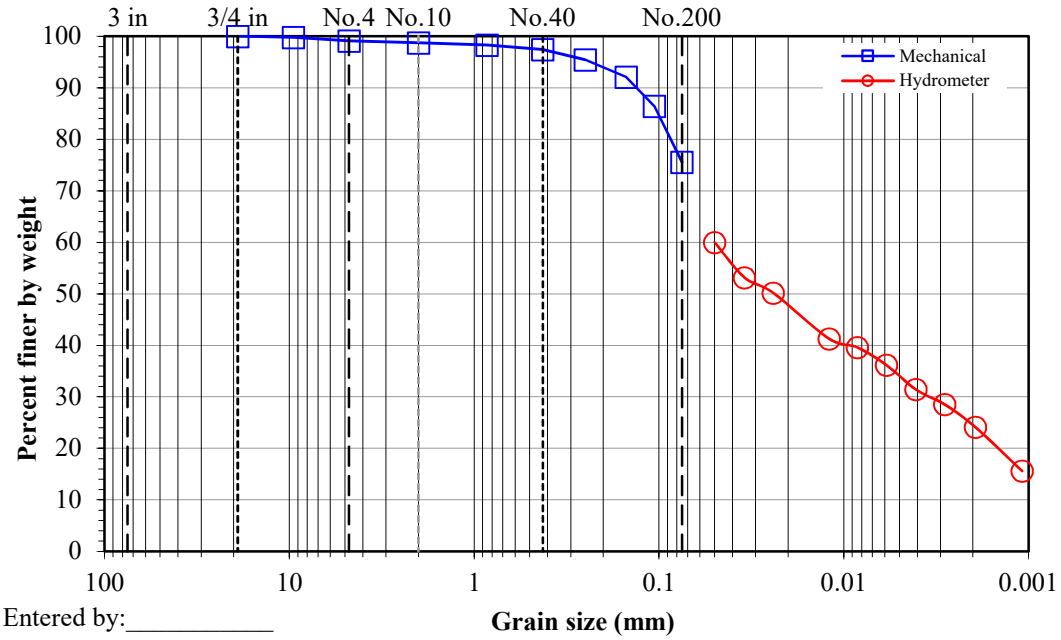
Water content data	C.F.1(+3/8")	S.F.1(-3/8")	Hyd.(-No.10)
Moist soil + tare (g):	147.79	435.66	25.99
Dry soil + tare (g):	147.33	392.51	23.75
Tare (g):	127.04	126.83	7.50
Water content (%):	2.27	16.24	13.78

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.7	Assumed	
Bulb No.	6	Hyd. fraction:	98.76
Cylinder ID:	N3	Dispersion device:	Air-jet

Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer
6"		150	-
4"		100	-
3"		75	-
1.5"		37.5	-
1"		25	-
3/4"		19	100.0
3/8"	17.92	9.5	99.8
No.4	1.82	4.75	99.1
No.10	2.71	2	98.8
No.20	3.99	0.85	98.3
No.40	6.25	0.425	97.4
No.60	11.55	0.25	95.4
No.100	20.49	0.15	92.1
No.140	35.48	0.106	86.5
No.200	64.53	0.075	75.5

	Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
	1	22	40	0.0500	59.99
	2	22	36	0.0345	53.17
	4	22	34.25	0.0241	50.18
	15	22.1	29	0.0120	41.30
	30	22.1	28	0.0084	39.59
	60	22.1	26	0.0059	36.18
	120	22	23.25	0.0041	31.43
	240	22.1	21.5	0.0028	28.51
	494	21.9	19	0.0019	24.12
<=1st Split	1458	21.9	14	0.0011	15.60
<=Split hyd.					

Gravel (%): 0.9
Sand (%): 23.6
Fines (%): 75.5



Entered by:_____
Reviewed:_____

Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: JAB/EH/BRR

Boring No.:
Sample: B3TP-2
Depth: 0-15'
Description: Sandy lean CLAY, brown

ASTM Standard(s)	ASTM D6913 and ASTM D7928	
Split:	Yes	
First Split sieve:	3/8"	
Second split:	No	
	Moist	Dry
Total sample wt. (g):	8606.1	7443.4
+3/8" Coarse fraction (g):	4.08	3.99
-3/8" Split fraction (g):	245.05	211.93
Hydrometer fraction (g):	65.51	57.75
First Split fraction:	0.999	

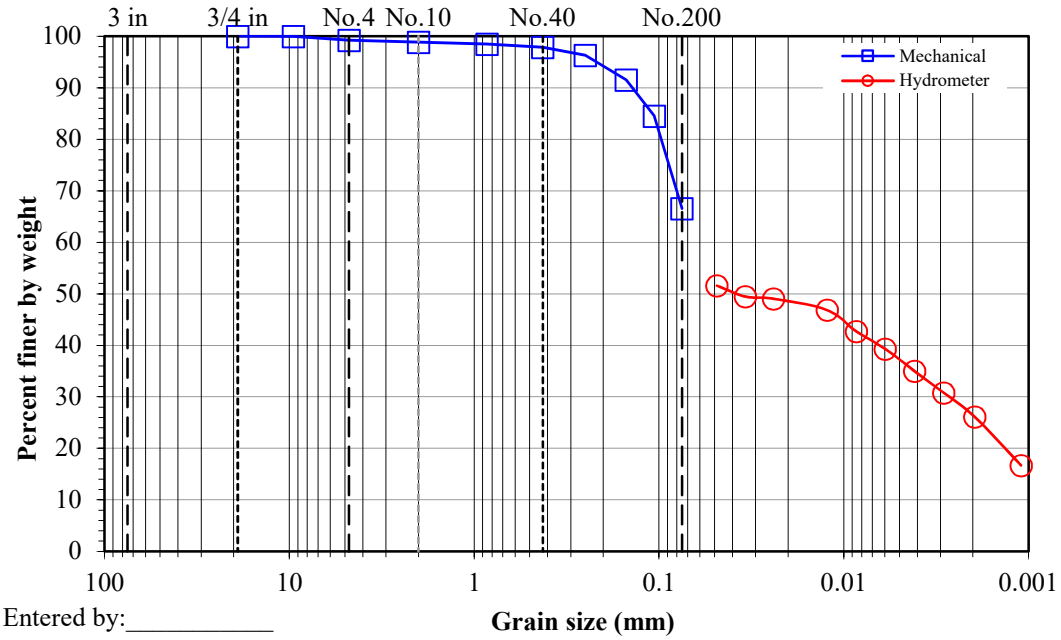
Water content data	C.F.1(+3/8")	S.F.1(-3/8")	Hyd.(-No.10)
Moist soil + tare (g):	132.04	373.55	35.45
Dry soil + tare (g):	131.95	340.43	32.09
Tare (g):	127.91	128.50	7.10
Water content (%):	2.23	15.63	13.45

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.7	Assumed	
Bulb No.	6	Hyd. fraction:	98.84
Cylinder ID:	N10	Dispersion device:	Air-jet

Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer
6"		150	-
4"		100	-
3"		75	-
1.5"		37.5	-
1"		25	-
3/4"		19	100.0
3/8"	3.99	9.5	99.9
No.4	1.58	4.75	99.2
No.10	2.34	2	98.8
No.20	3.08	0.85	98.5
No.40	4.48	0.425	97.8
No.60	7.69	0.25	96.3
No.100	17.86	0.15	91.5
No.140	32.70	0.106	84.5
No.200	70.82	0.075	66.5

	Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
	1	22.1	35.25	0.0486	51.58
	2	22.1	34	0.0341	49.46
	4	22.1	33.75	0.0240	49.04
	15	22	32.5	0.0123	46.86
	30	22.1	30	0.0086	42.69
	60	22.1	28	0.0060	39.31
	120	22	25.5	0.0041	35.02
	240	22	23	0.0029	30.78
	497	22	20.25	0.0020	26.13
<=1st Split	1450	21.1	15	0.0011	16.70
<=Split hyd.					

Gravel (%): 0.8
Sand (%): 32.7
Fines (%): 66.5



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Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/9/2020
By: JAB/EH/BRR

Boring No.:
Sample: B3TP-2
Depth: 15-25'
Description: Sandy lean CLAY, brown

ASTM Standard(s)	ASTM D6913 and ASTM D7928	
Split:	Yes	
First Split sieve:	3/8"	
Second split:	No	
	Moist	Dry
Total sample wt. (g):	8940.3	7903.2
+3/8" Coarse fraction (g):	56.99	54.10
-3/8" Split fraction (g):	261.81	231.33
Hydrometer fraction (g):	61.80	55.36
First Split fraction:	0.993	

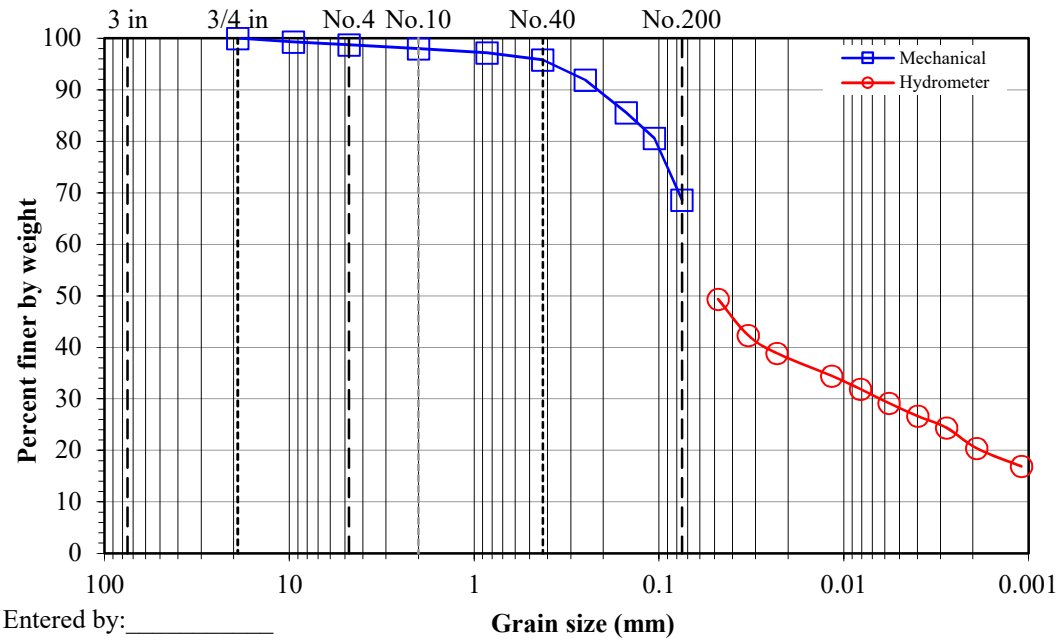
Water content data	C.F.1(+3/8")	S.F.1(-3/8")	Hyd.(-No.10)
Moist soil + tare (g):	192.03	389.49	28.99
Dry soil + tare (g):	188.63	359.01	26.74
Tare (g):	125.02	127.68	7.41
Water content (%):	5.35	13.18	11.64

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.7	Assumed	
Bulb No.	6	Hyd. fraction:	98.00
Cylinder ID:	T3	Dispersion device:	Air-jet

Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer	
6"		150	-	
4"		100	-	
3"		75	-	
1.5"		37.5	-	
1"		25	-	
3/4"		19	100.0	
3/8"	54.10	9.5	99.3	<=1st Split
No.4	1.37	4.75	98.7	
No.10	3.07	2	98.0	<=Split hyd.
No.20	4.94	0.85	97.2	
No.40	8.21	0.425	95.8	
No.60	17.25	0.25	91.9	
No.100	32.03	0.15	85.6	
No.140	43.64	0.106	80.6	
No.200	71.52	0.075	68.6	

Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
1	22	33	0.0478	49.34
2	22	29	0.0329	42.34
4	22	27	0.0229	38.84
15	22	24.5	0.0116	34.46
30	22.1	23	0.0081	31.90
60	21.9	21.5	0.0057	29.15
120	22.1	20	0.0040	26.65
240	22	18.75	0.0028	24.40
492	21.9	16.5	0.0019	20.40
1443	21.9	14.5	0.0011	16.90

Gravel (%): 1.3
Sand (%): 30.1
Fines (%): 68.6



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Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: JAB/BRR/EH

Boring No.:
Sample: B3TP-3
Depth: 0-15'
Description: Silty SAND, brown

ASTM Standard(s)	ASTM D6913 and ASTM D7928
Split:	Yes
First Split sieve:	3/8"
Second split:	No

	Moist	Dry
Total sample wt. (g):	9290.9	8833.5
+3/8" Coarse fraction (g):	47.94	46.11
-3/8" Split fraction (g):	212.21	201.75
Hydrometer fraction (g):	83.68	80.77
First Split fraction:	0.995	

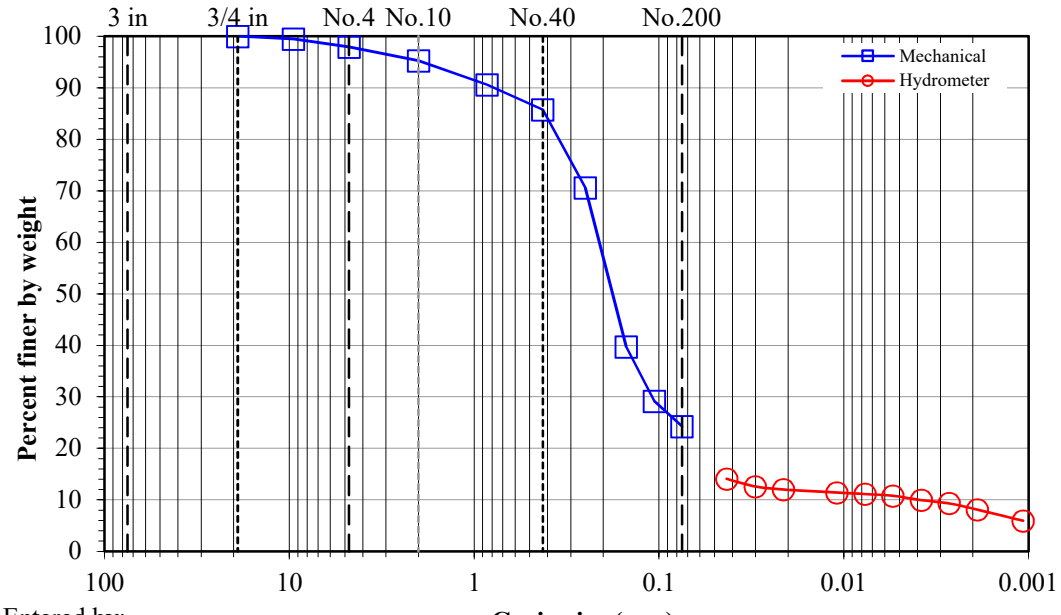
Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer	
6"		150	-	
4"		100	-	
3"		75	-	
1.5"		37.5	-	
1"		25	-	
3/4"		19	100.0	
3/8"	46.11	9.5	99.5	<=1st Split
No.4	3.18	4.75	97.9	
No.10	8.56	2	95.3	<=Split hyd.
No.20	18.00	0.85	90.6	
No.40	27.85	0.425	85.7	
No.60	58.55	0.25	70.6	
No.100	121.17	0.15	39.7	
No.140	142.63	0.106	29.2	
No.200	152.51	0.075	24.3	

Water content data	C.F.1(+3/8")	S.F.1(-3/8")	Hyd.(-No.10)
Moist soil + tare (g):	171.32	339.33	49.45
Dry soil + tare (g):	169.45	328.87	48.17
Tare (g):	122.41	127.12	12.64
Water content (%):	3.98	5.18	3.60

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.65	Assumed	
Bulb No.	6	Hyd. fraction:	95.26
Cylinder ID:	N16	Dispersion device:	Air-jet

Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
1	22	16.75	0.0430	14.08
2	22	15.5	0.0301	12.60
4	22	15	0.0212	12.01
15	22	14.5	0.0109	11.42
30	22	14.25	0.0077	11.13
60	21.9	14	0.0054	10.79
120	22	13.25	0.0038	9.95
239	21.9	12.75	0.0027	9.32
474	21.8	11.75	0.0019	8.10
1430	21.6	10	0.0011	5.95

Gravel (%): 2.1
Sand (%): 73.6
Fines (%): 24.3



Entered by:_____
Reviewed:_____

Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis

(In general accordance with ASTM D6913 and ASTM D7928)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: JP/JAB/EH/BRR

Boring No.:
Sample: B3TP-3
Depth: 15-30'
Description: Lean CLAY with sand, brown

ASTM Standard(s): ASTM D6913 and ASTM D7928
Split: No
Second split: No

	Moist	Dry
Total sample wt. (g):	260.92	217.49
Hydrometer fraction (g):	65.96	56.18

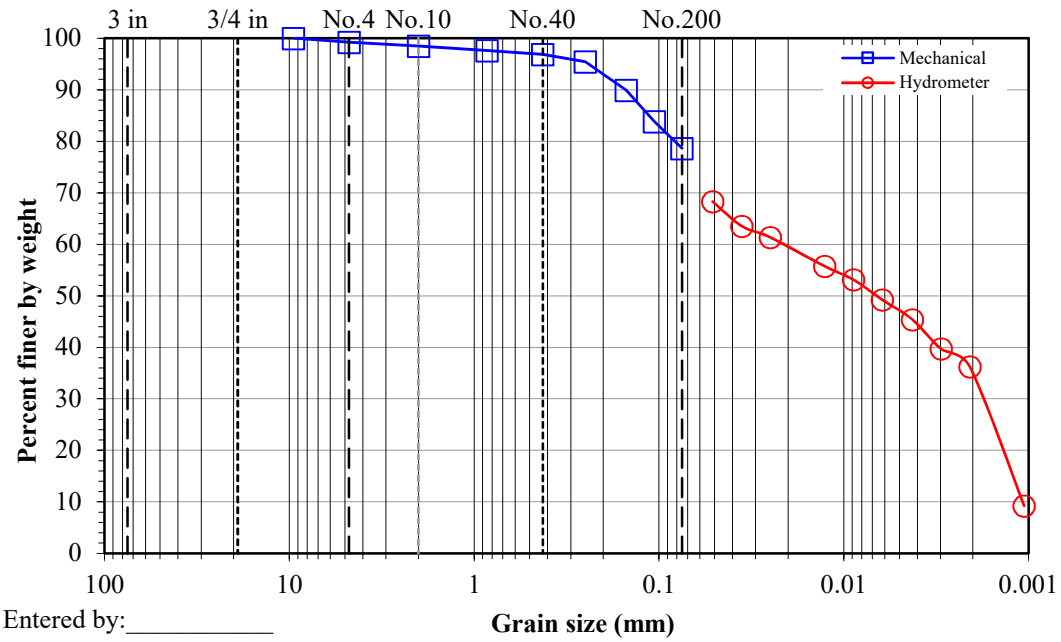
Water content data		S.F.	Hyd.(-No.10)
Moist soil + tare (g):		384.53	47.86
Dry soil + tare (g):		341.10	42.64
Tare (g):		123.61	12.66
Water content (%):		19.97	17.41

Hydrometer data			
Hyd. split:	No.10		
Gs:	2.7	Assumed	
Bulb No.	6	Hyd. fraction:	98.48
Cylinder ID:	N18	Dispersion device:	Air-jet

Sieve	Accum. Wt. Ret. (g)	Grain Size (mm)	Percent Finer
6"		150	-
4"		100	-
3"		75	-
1.5"		37.5	-
1"		25	-
3/4"		19	-
3/8"		9.5	100.0
No.4	1.74	4.75	99.2
No.10	3.31	2	98.5
No.20	5.15	0.85	97.6
No.40	6.80	0.425	96.9
No.60	9.89	0.25	95.5
No.100	21.91	0.15	89.9
No.140	35.12	0.106	83.9
No.200	46.50	0.075	78.6

	Elapsed time (min)	Temp. (°C)	Hydrometer Reading	Grain Size (mm)	% Soil in Suspension
	1	21.9	44.25	0.0512	68.30
	2	21.9	41.5	0.0357	63.53
	4	21.9	40.25	0.0250	61.36
	15	21.9	37	0.0127	55.73
	30	21.9	35.5	0.0089	53.13
	60	21.9	33.25	0.0062	49.23
	123	22	31	0.0043	45.39
	240	21.9	27.75	0.0030	39.70
	478	21.9	25.75	0.0021	36.23
<=Split hyd.	1434	21.7	10.25	0.0011	9.24

Gravel (%): 0.8
Sand (%): 20.6
Fines (%): 78.6



Entered by:_____
Reviewed:_____

Classification of Soils for Engineering Purposes

(ASTM D2487)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: BRR

Sample Info.									
	Boring No.								
	Sample:	B3TP-1	B3TP-1	B3TP-2	B3TP-2	B3TP-3	B3TP-3		
	Depth:	0-10'	10-20'	0-15'	15-25'	0-15'	15-30'		
Liquid Limit (%):		29	34	31	30	NP	29		
Plastic Limit (%):		17	15	14	15	NP	15		
Plastic Index (%):		12	19	17	15	NP	14		
Gravel (%):		3.3	0.9	0.8	1.3	2.1	0.8		
Sand (%):		69.3	23.6	32.7	30.1	73.6	20.6		
Fines (%):		27.4	75.5	66.5	68.6	24.3	78.6		
D_{60} (mm):									
D_{30} (mm):									
D_{10} (mm):									
Cu:									
Cc:									
Group Symbol:		**SC**	**CL**	**CL**	**CL**	**SM**	**CL**		
Group Name:		**Clayey SAND**	**Lean CLAY with sand**	**Sandy lean CLAY**	**Sandy lean CLAY**	**Silty SAND**	**Lean CLAY with sand**		

Entered by:_____

Reviewed:_____

Moisture, Ash, and Organic Matter of Peat and Other Organic Soils

(ASTM D2974)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 12/31/2019
By: BF/BSS/JAB

Sample Info.	Boring No.								
	Sample:	B1TP-1	B1TP-2	B1TP-3	B2TP-1	B2TP-2	B2TP-3	B3TP-1	
	Depth:	10-15'	10-20'	0-10'	20-25'	0-15'	12-15'	10-20'	
	Test Method:	C	C	C	C	C	C	C	
	Furnace temp. (°C)	440	440	440	440	440	440	440	
Moisture	Wet soil + tare (g)	680.76	630.70	611.32	614.17	599.84	552.15	569.66	
	Dry soil + tare (g)	653.03	624.18	585.74	578.80	580.49	525.90	536.95	
	Tare (g)	380.50	375.01	374.87	378.48	374.28	380.26	341.22	
Ash / Organic Info	Mass of crucible and oven-dried sample (g)	653.03	624.18	585.74	578.80	580.49	525.90	536.95	
	Mass of crucible and ash (g)	648.81	622.08	584.01	572.54	578.24	521.82	530.70	
	Mass of crucible (g)	380.50	375.01	374.87	378.48	374.28	380.26	341.22	
Moisture Content, w (%) [a]		**10.2**	**2.6**	**12.1**	**17.7**	**9.4**	**18.0**	**16.7**	
Ash Content (%)		**98.5**	**99.2**	**99.2**	**96.9**	**98.9**	**97.2**	**96.8**	
Organic Matter (%)		**1.5**	**0.8**	**0.8**	**3.1**	**1.1**	**2.8**	**3.2**	

[a] Moisture contents are by proportion of oven-dried mass (geotechnical convention).

Entered by:_____
Reviewed:_____

DETERMINING DISPERSIVE CHARACTERISTICS OF CLAYEY SOILS BY THE CRUMB TEST

(ASTM D6572)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: JP

Boring No.:
Sample: B3TP-1
Depth: 10-20'
Sample Description: Lean CLAY with sand, brown
Engineering Classification: CL
Specimen Type: Natural irregularly shaped crumb

Specific Gravity, Gs:	2.65	Assumed
Curing Time:	0	minutes
Water used:	Distilled	
Water content:	Air-dried	
Wet soil + tare (g)	142.03	
Dry soil + tare (g)	139.46	
Tare (g)	123.63	
Water content, w (%)	16.2	

Initial water temperature: 19.0 °C
Date test started: 12/27/2019
Time at beginning of test: 10:22

Specimen Number	2 minutes		1 hour		6 hours	
	Grade	Temp. (°C)	Grade	Temp. (°C)	Grade	Temp. (°C)
1	1	19.0	1	18.4	1	18.0
2	1	19.0	1	18.4	1	18.0

Dispersive classification: Grade 1-Nondispersive

Entered: _____
Reviewed: _____

DETERMINING DISPERSIVE CHARACTERISTICS OF CLAYEY SOILS BY THE CRUMB TEST

(ASTM D6572)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: JP

Boring No.:
Sample: B3TP-2
Depth: 15-25'
Sample Description: Sandy lean CLAY, brown
Engineering Classification: CL
Specimen Type: Natural irregularly shaped crumb

Specific Gravity, Gs:	2.65	Assumed
Curing Time:	0	minutes
Water used:	Distilled	
Water content:	Air-dried	
Wet soil + tare (g)	178.14	
Dry soil + tare (g)	169.56	
Tare (g)	114.72	
Water content, w (%)	15.6	

Initial water temperature: 18.9 °C
Date test started: 12/27/2019
Time at beginning of test: 10:24

Specimen Number	2 minutes		1 hour		6 hours	
	Grade	Temp. (°C)	Grade	Temp. (°C)	Grade	Temp. (°C)
1	1	18.9	1	18.3	1	18.0
2	1	18.9	1	18.3	1	18.0

Dispersive classification: Grade 1-Nondispersive

Entered: _____
Reviewed: _____

Laboratory Compaction Characteristics of Soil

(ASTM D698 / D1557)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Closures; Delta, UT
Date: 1/10/2020
By: BSS

Sample: B3TP-1 & B3TP-2 & B3TP-3
Depth: 10-30'
Sample Description: Sandy lean CLAY, brown
Engineering Classification: CL
As-received water content (%): Not requested

Method: ASTM D698 B
Mold Id. Inc 3
Mold volume (ft^3): 0.0332

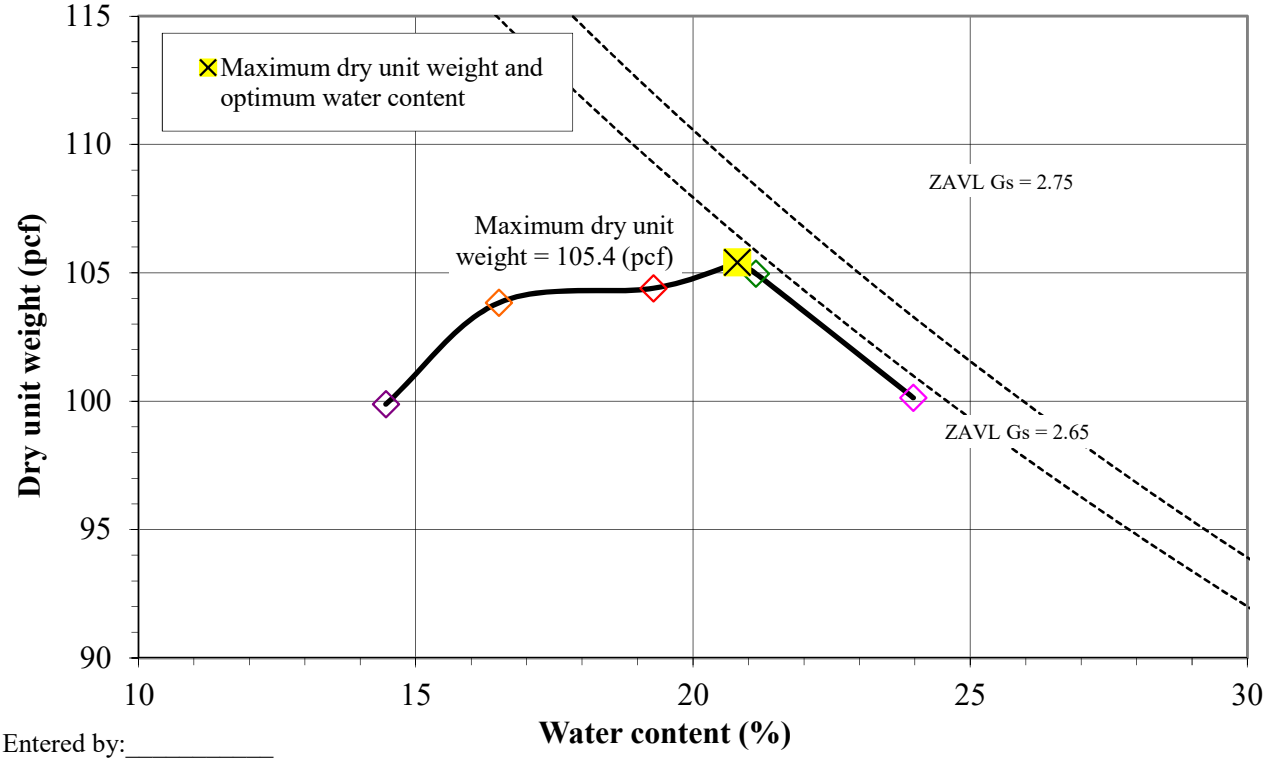
Preparation method: Moist
Rammer: Mechanical-circular face
Rock Correction: No

Optimum water content (%): 20.8
Maximum dry unit weight (pcf): 105.4

Point Number	-2%	+2%	+4%	+6%	As Is			
Wt. Sample + Mold (g)	5945.2	6099.0	6138.5	6093.1	6045.3			
Wt. of Mold (g)	4221.2	4221.2	4221.2	4221.2	4221.2			
Wet Unit Wt., γ_m (pcf)	114.3	124.5	127.2	124.1	121.0			
Wet Soil + Tare (g)	971.48	1138.75	1103.65	1005.38	941.90			
Dry Soil + Tare (g)	890.21	990.52	948.61	852.61	840.16			
Tare (g)	328.25	221.93	215.02	215.35	223.51			
Water Content, w (%)	14.5	19.3	21.1	24.0	16.5			
Dry Unit Wt., γ_d (pcf)	99.9	104.4	105.0	100.1	103.8			

Comments:

Test specimen consisted of material from B3TP-1 @ 10-20', B3TP-2 @ 15-25', and B3TP-3 @ 15-30'. Due to insufficient sample quantity, points +4%, +6%, and As Is contained previously compacted material .



Entered by:_____

Reviewed:_____

Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter, Method C (ASTM D5084)

Project: Stantec
No: M00287-022
Location: IPSC CCR Unit Clousres; Delta, UT
Date: 1/15/2020
By: EH

Boring No.:
Sample: B3TP-1, B3-TP-2, & B3TP-3
Depth: 10-30'
Sample Description: Sandy lean CLAY, brown
Sample Type: Laboratory Compacted
Compaction Specifications: 95 (%) Dry unit weight
at 20.8 (%) w
Optimum water content (%) 20.8
Maximum dry unit weight (pcf) 105.4
Gs 2.7 Assumed
Cell No. 2
Station No. 3
Permeant liquid used De-aired tap water
Total backpressure (psi) 35
Effective horiz. consolidation stress (psi) 3
Effective vert. consolidation stress (psi) 3

	Initial (o)	Final (f)
Sample Height, H (in)	2.994	2.988
Sample Diameter, D (in)	2.413	2.400
Sample Length, L (cm)	7.605	7.589
Sample Area, A (cm^2)	29.503	29.195
Sample Volume, V (cm^3)	224.37	221.55
Wt. Rings + Wet Soil (g)	435.45	452.38
Wt. Rings (g)	0	0
Wet Unit Wt., γ_m (pcf)	121.2	127.5
Wet Soil + Tare (g)	292.31	578.61
Dry Soil + Tare (g)	263.64	486.29
Tare (g)	127.12	127.15
Weight of solids, Ws (g)	359.87	359.87
Water Content, w (%)	21.00	25.71
Dry Unit Wt, γ_d (pcf)	100.1	101.4
Void ratio, e, for assumed Gs	0.68	0.69
Saturation (%), for assumed Gs	83.0	100 [a]
Average K[b] (cm/sec)	**1.5E-05**	

[a] Saturation set to 100% for phase calculations
[b] K corrected to 20ºC

	Initial (o)	Final (f)
B value	0.58	0.96
External Burette (cm^3)	14.90	23.70
Cell Pressure (psi)	0.0	38.0

Backpressure bottom (psi) 35.0
Backpressure top (psi) 35.0
System volume coefficient (cm^3/psi) 0.158
System volume change (cm^3) 5.99
Net sample volume change (cm^3) -2.81
Bottom burette ground length, l_b (cm) 82.25
Top burette ground length, l_t (cm) 81.95
Burette area, a (cm^2) 0.197
Conversion, reading to cm head (cm/rd) 5.076

Start Date and Time: 1/14/20 16:34

Elapsed time (sec)	Bottom Burette (cm^3)	Top Burette (cm^3)	h_1 (cm)	h_2 (cm)	K (cm/sec)	Temp (ºC)	Visc. Ratio R_T	K[b] (cm/sec)
30.0	1.21 1.27	8.66 8.59	38.14	37.46	1.5E-05	23.5 23.5	0.92	1.4E-05
30.0	1.27 1.34	8.59 8.52	37.46	36.75	1.6E-05	23.5 23.5	0.92	1.5E-05
30.0	1.34 1.41	8.52 8.45	36.75	36.06	1.6E-05	23.5 23.5	0.92	1.5E-05
30.0	1.41 1.47	8.45 8.38	36.06	35.38	1.6E-05	23.5 23.5	0.92	1.5E-05
30.0	1.47 1.55	8.38 8.31	35.38	34.61	1.9E-05	23.5 23.5	0.92	1.7E-05

Comments:
Test specimen was remolded to 95% of ASTM D698 B (which inlcuded combined material from B3TP-1 @ 10-20', B3TP-2 @ 15-25', and B3TP-3 @ 15-30') at optimum water content. Test specimen comprised of combined material.

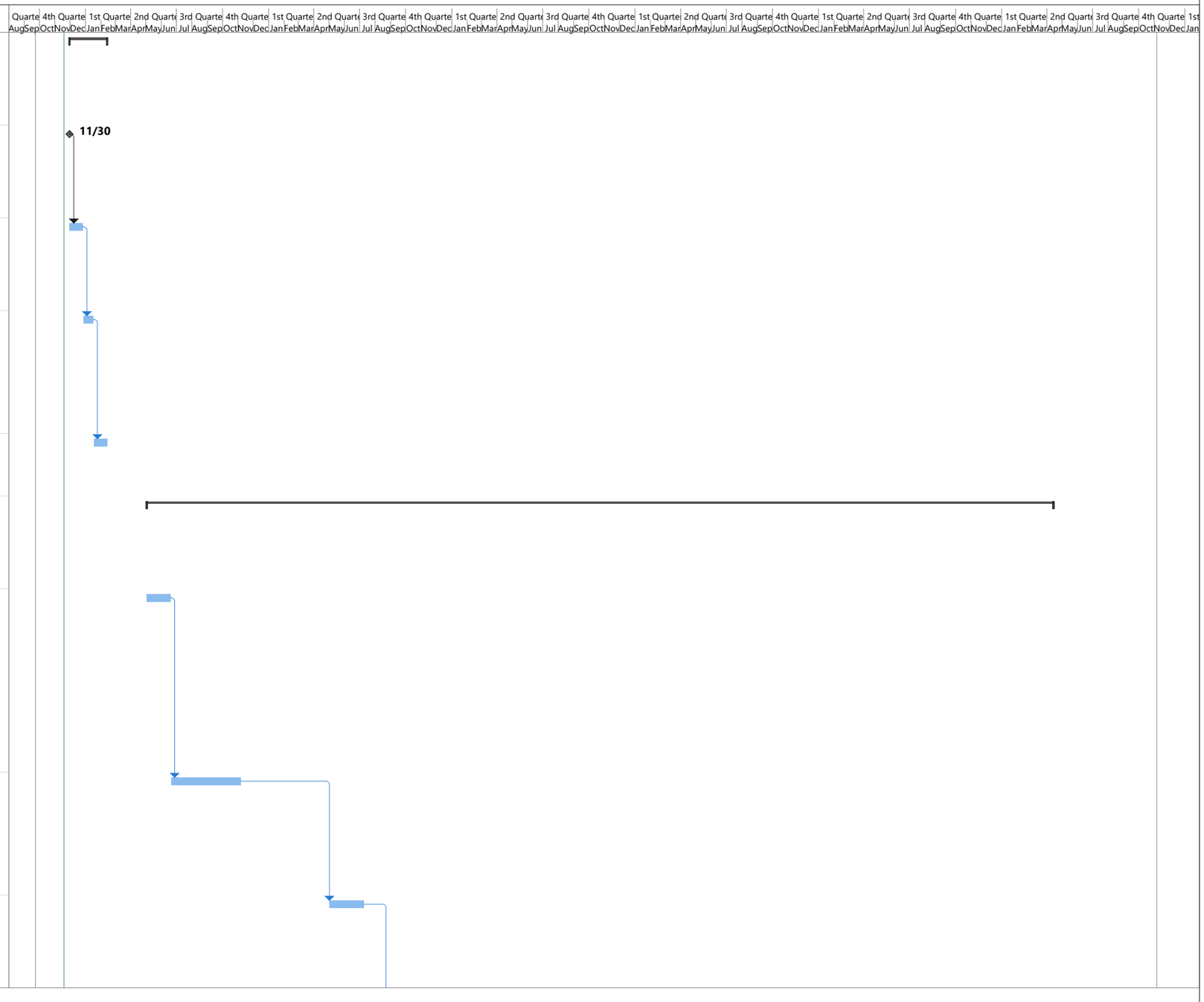
Entered by:_____
Reviewed:_____

Appendix E

Closure Schedule

Wawater Basin Closure Schedule

ID	Task Mode	Task Name	Duration	Start	Finish	Predecessors	Resource Names
1		**Closure Plan**	**55 days**	**Mon 11/30/2**	**Fri 2/12/21**		
2		Submit WW Basin Closure Plan to UDEQ	0 days	Mon 11/30/20	Mon 11/30/20		
3		UDEQ Review	20 days	Mon 11/30/2	Fri 12/25/20	2	
4		Revise and Submit WW Basin Clsoure Plan per UDEQ Review	15 days	Mon 12/28/20	Fri 1/15/21	3	
5		UDEQ Approval of Closure Plan	20 days	Mon 1/18/21	Fri 2/12/21	4	
6		**WW Basin Initial Fill Placement and Dewatering**	**1291 days**	**Mon 5/3/21**	**Mon 4/13/26**		
7		Step 1 Divider Berm Construction (including decanting of pond)	35 days	Mon 5/3/21	Fri 6/18/21		
8		Step 2 Southern Cell Fill Placement (1st Season)	100 days	Mon 6/21/21	Fri 11/5/21	7	
9		Step 2 Southen Fill Placement (2nd Season)	50 days	Mon 5/2/22	Fri 7/8/22	8	



Project: WW Basin Closure Sche
Date: Thu 11/19/20

Task	Summary	Inactive Milestone	Duration-only	Start-only	External Milestone	Manual Progress
Split	Project Summary	Inactive Summary	Manual Summary Rollup	Finish-only	Deadline	
Milestone	Inactive Task	Manual Task	Manual Summary	External Tasks	Progress	

Wawater Basin Closure Schedule

ID	Task Mode	Task Name	Duration	Start	Finish	Predecessors	Resource Names
10		Step 2 Southern Cell Dewatering	70 days	Mon 5/2/22	Fri 8/5/22	9FF+20 days	
11		Cease Flows to Northern Portion of WW Basin	0 days	Tue 7/1/25	Tue 7/1/25		
12		Step 3 Northern Fill Placement and Dewatering	85 days	Tue 7/15/25	Mon 11/10/25	11FS+10 days	
13		Final Dewatering of Basin Solids	100 days	Tue 11/11/25	Mon 3/30/26	12	
14		Final Fill Placement	10 days	Tue 3/31/26	Mon 4/13/26	13	
15		**Final Cover Installation**	**135 days**	**Fri 5/1/26**	**Thu 11/5/26**		
16		Liner Installation	45 days	Fri 5/1/26	Thu 7/2/26	14	
17		Cover Installation	58 days	Fri 7/3/26	Tue 9/22/26	16	
18		Cover Seeding	5 days	Fri 10/30/26	Thu 11/5/26	17	

Project: WW Basin Closure Sche
Date: Thu 11/19/20

Task	Summary	Inactive Milestone	Duration-only	Start-only	External Milestone	Manual Progress
Split	Project Summary	Inactive Summary	Manual Summary Rollup	Finish-only	Deadline	
Milestone	Inactive Task	Manual Task	Manual Summary	External Tasks	Progress	